



NASA Technical Paper 3512

Buckling of Composite Beams (CDDF Final Report—Project No. 91-20)

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October 1994



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TABLE OF CONTENTS

| | Page |
|---|------|
| INTRODUCTION | 1 |
| APPROACH..... | 3 |
| MANUFACTURING METHODS AND TESTING..... | 4 |
| EFFECT OF FIBER ORIENTATION OF FLANGE DEFLECTION..... | 5 |
| EFFECT OF FIBER ORIENTATION OF SHEAR CENTER LOCATION..... | 9 |
| CONCLUSIONS | 14 |
| REFERENCES..... | 15 |
| APPENDIX | 17 |

LIST OF ILLUSTRATIONS

| Figure | Title | Page |
|--------|--|------|
| 1. | Open-section beam | 2 |
| 2. | Load application points..... | 3 |
| 3. | C-channel beams..... | 4 |
| 4. | Deflection gauge locations for composite beam load tests | 5 |
| 5. | Deflections for C-channel loaded at geometric center (AS4/3501-6)..... | 6 |
| 6. | Deflections for C-channel loaded at theoretical shear center (AS4/3501-6) | 7 |
| 7. | Deflections for C-channel loaded at geometric center (IM7/8552) | 8 |
| 8. | Deflections for C-channel loaded at theoretical shear center (IM7/8552)..... | 8 |
| 9. | Theoretical versus measured shear center comparison | 10 |
| 10. | Angle of twist versus load application points for Form 1 beam (AS4/3501-6).... | 11 |
| 11(a). | Angle of twist versus load application points for Form 1 beam with minimal load applied, $P \leq 5.0$ lb (AS4/3501-6)..... | 12 |
| 11(b). | Angle of twist versus load application points for Form 1 beam with minimal load applied, $P \leq 5.0$ lb (AS4/3501-6)..... | 13 |

LIST OF SYMBOLS

| | |
|----------|---|
| b | beam width, inches |
| h | beam height, inches |
| t_w | web thickness, inches |
| t_f | flange thickness, inches |
| P | load applied, lb |
| E | modulus of elasticity, lb/in ² |
| d | deflection, inches |
| ϕ | angle of twist, degrees |
| Θ | ply orientation angle, degrees |
| m, n | laminate stacking sequence number |

TECHNICAL PAPER

BUCKLING OF COMPOSITE BEAMS

INTRODUCTION

Torsion/bending coupling is an interesting, inherent phenomenon occurring in open-section structural elements subjected to bending and/or combined axial/bending loads. The benefits of using open-section beams are many: stackability, weight savings, and dual functions (e.g., serving as load carriers as well as conduits). Likewise, composites offer advantages over their metallic counterparts; primarily, their higher strength-to-weight ratios and lower coefficients of thermal expansion (CTE) are favorable. This type beam is used extensively in the aircraft industry in the fuselage support structure and wing supports, and has present and potential future use in aerospace applications; namely tankage stiffeners, intertank stiffeners, and as primary load carriers in truss work. However, in each of the aforementioned applications the layups are symmetric in nature. By understanding the bending/torsion effects indigenous to open-section beams one can make use of the tailorability of composites (to achieve desired optimum material properties) without compromise to the design (e.g., no added plies to accomplish symmetry).

Antisymmetric layups lend themselves well to thin-walled structural members. With an average ply thickness of 0.005 in, a beam of wall thickness less than 0.040 in (8 plies) is difficult to manufacture using a symmetric laminate layup.

Even an open-section beam made of an isotropic material has an inherent tendency to bend and twist if the transverse load is applied anywhere other than along the plane of the shear center. A transverse load applied to such open sections undergoes pure bending only when the load is applied at points along the shear center located outside the section.¹ As Valsov illustrates, an open section not loaded at points along its shear center will twist and bend as in figure 1(b). Figure 1(a) shows the same open-section beam with no load applied. In the case with loading at the shear center, notice that pure bending results, as in figure 1(c).

As in symmetrically laminated structural elements, unsymmetrically laminated elements can have many different resultant properties depending on the chosen material system, ply orientations, and layup sequence.² Add these factors to the bending/twisting coupling inherent to an open section loaded transversely and one can easily surmise the difficulties that might be associated with designing using unsymmetric, open-section structural elements.

Characterization of the relationship between angle of twist and ply orientation is one of the objectives of this paper. This is being done based strictly on test results, with results of the analytical aspect to be presented in a later paper. Also examined and presented here are the results of testing involving shear center location, load application relative to shear center, and the resulting angle of twist on open-section beams. Curves depicting these parameters and the various interrelationships are contained in this paper.

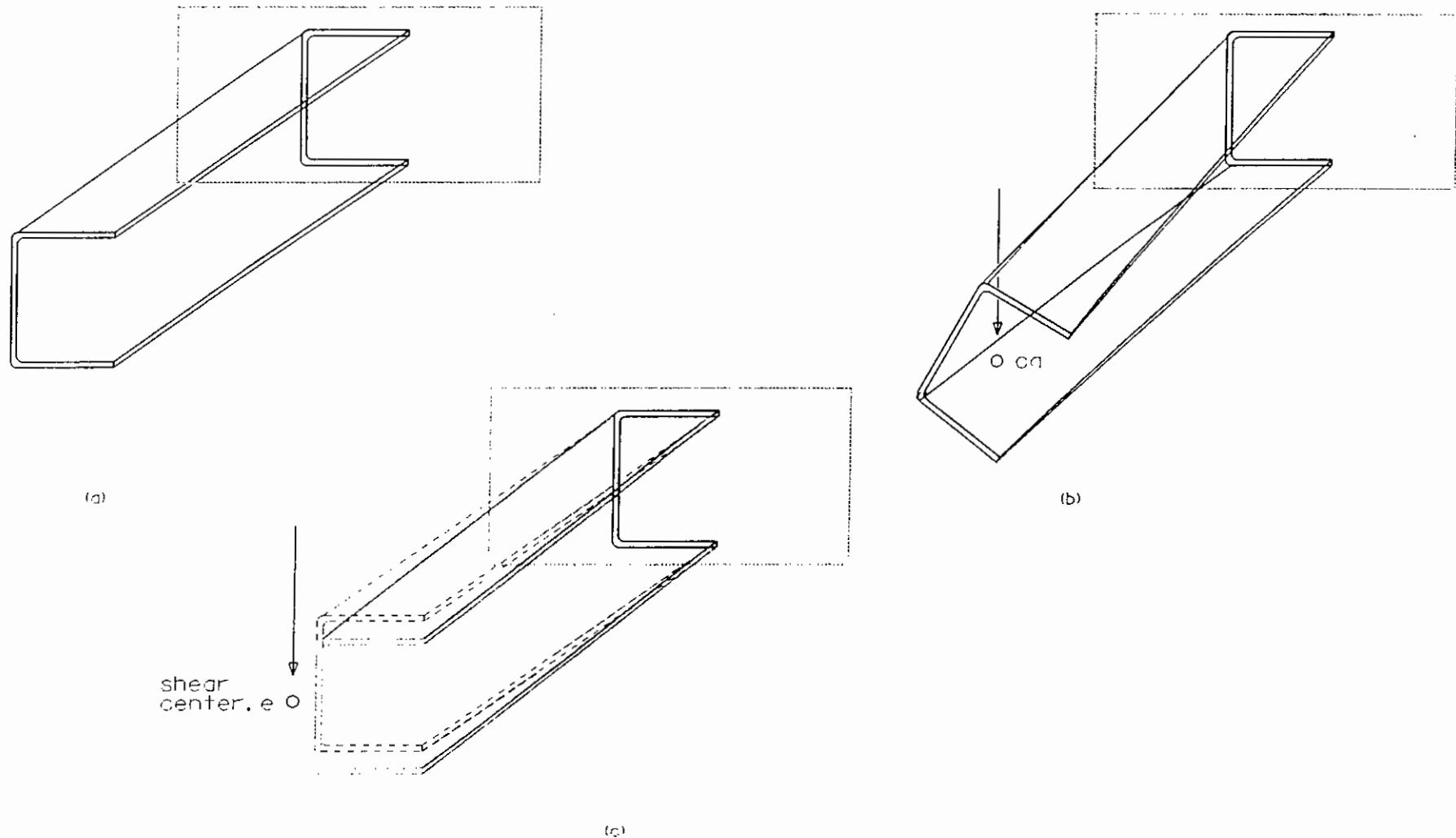


Figure 1. Open-section beam.

APPROACH

To reach the first objective (characterization of the relationship between the angle of twist and ply orientation), the effects of distortions due to CTE must be included. This produces an amount of pretwist in the beam apart from the twisting that is to occur due to the load not being applied at the actual shear center. Examination of the various laminate layups will reveal a general relationship between the ply orientation and angle of twist.

Open-section beams of various ply orientations were manufactured and tested to determine what effect ply orientation and laminate layup have on the shear center location, i.e., can the shear center in open-section structural elements be shifted toward its geometric center? As illustrated in figure 2, the geometric center and theoretical shear center differ. Yet this moving of the shear center can possibly be observed by comparing the various twist and deflections in each beam tested as a specified load is applied at designated positions along the horizontal plane of the shear center (fig. 2). Recalling that no twist (pure bending only) will occur when transverse loads are applied at the shear center, one can better define the location of the "true" shear center. This "true" position, since it is observed after the curing process (hence, CTE mismatches are already accounted for), reflects the dependency of shear center on laminate layup. No attempts were made to quantify this preload twist; however, measurements were taken prior to testing the beams to analyze trends.

Open-section beams (C-channels) of the layup $[0/\Theta]_n$ are referred to as Form 1. The laminates referred to as Form 2 are of $([0/\Theta]_n, [0/\Theta]_n)$ construction.

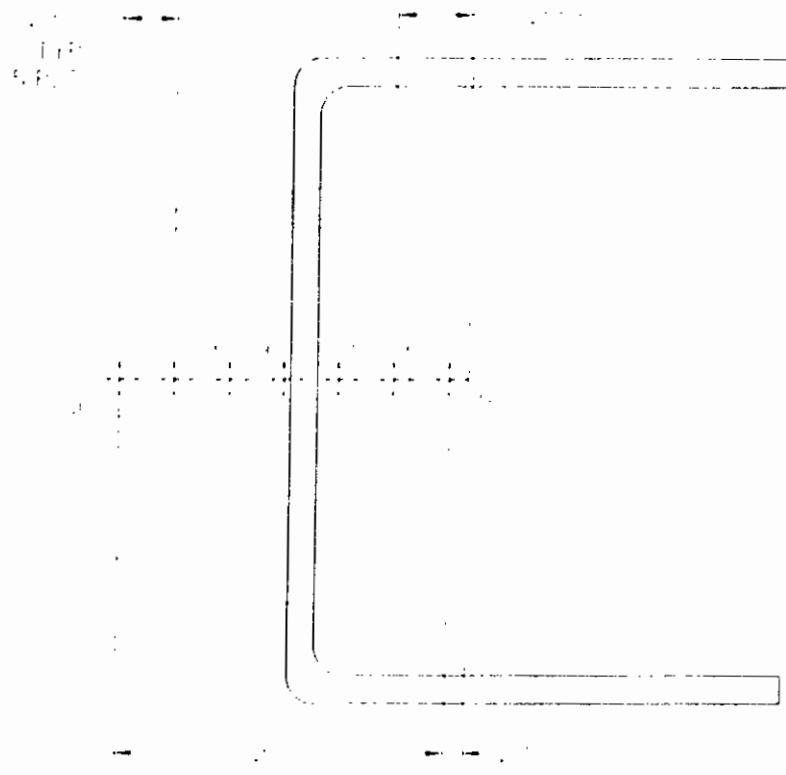
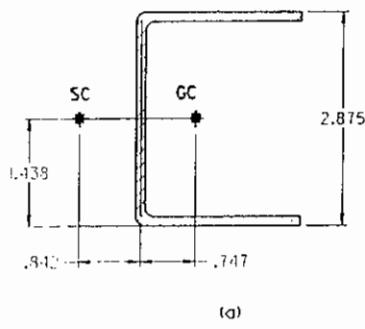


Figure 2. Load application points.

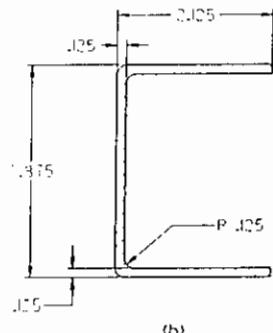
MANUFACTURING METHODS AND TESTING

As previously stated, the beams were fabricated using the hand layup and hot-drape forming techniques. Hand layup involved cutting 3-in wide strips of the unidirectional tape to the correct lengths and wrapping the tape at proper angles across the tool (in this case a solid aluminum square tube served as the tool) layer by layer. Intermediate vacuum bagging was performed to ensure adequate compaction prior to the autoclave cure of the part. Hot-drape forming was a new procedure used to take advantage of the multiaxis tape laying machine. Using this technique panels (approximately 18-in wide by 72-in long) were tape laid by machine. This provided repeatability of the part, provided adequate compaction (tape head is pressure sensitive, i.e., constant pressure as tape is laid in place), and reduced time of manufacturing. After the panels were cut to a strip 9 by 72 in, they were "draped" over the same tool used previously, bagged, and placed in the hot-drape forming "oven" under controlled temperatures (see the photographs in the appendix). This allowed the material's resin to flow enough so that the panel would take the shape of the tool.

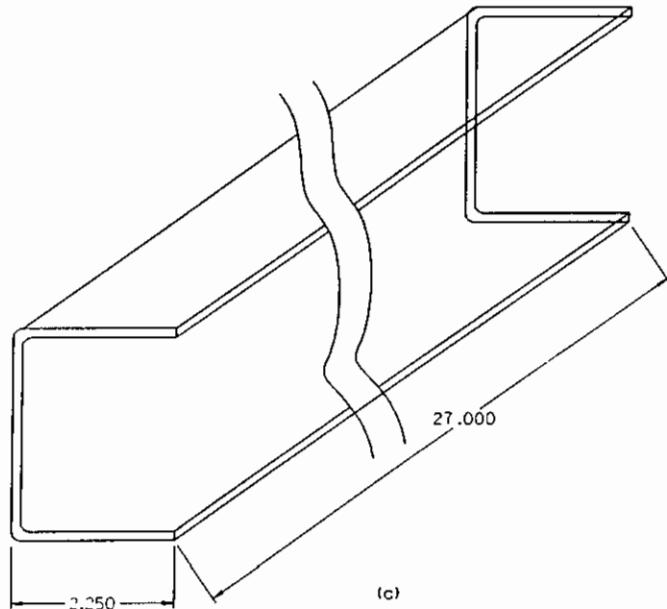
After the part was formed (regardless of method), it was vacuum bagged for autoclave cure using standard procedures. The resulting channels were then removed from the tool and cut and machined to dimensions suitable for testing. Figure 3 shows the geometry of the finished beam prior to instrumentation for testing.



(a)



(b)



C-CHANNEL COMPOSITE BEAM

C-CHANNEL CROSS-SECTION

Figure 3. C-channel beams.

The testing included applying load at several locations along the horizontal plane of the shear and geometric centers. After the beam was instrumented and the load cells calibrated, load was applied in 15 to 20 lb increments until a total of 130 lb was being applied. Beginning at the theoretical shear center, this procedure was repeated (at 0.25-in increments) until the final measurements at the geometric center (fig. 2). During the test procedure, the deflections and strains were recorded with the addition of load at each of the incremental points.

EFFECT OF FIBER ORIENTATION OF FLANGE DEFLECTION

Graphs 1 through 6 of section I of the appendix show deflections in the upper and lower flanges for samples of various layups of the C-channels. Also, charts 1 through 6 show a complete set of deflection data for the $[0/75]_{12}$ case. The beams were instrumented as shown in figure 4. An interesting occurrence in the beams, discovered through observation of the test results, is that the deflections decreased as the load application point is moved away from the center of gravity. This fact becomes more important in the following section when angle of twist is discussed. As expected, the deflection represented by D9 on the charts is smallest; this measurement is taken at a point on the beam's web. The top flange deflected the most in all cases examined.

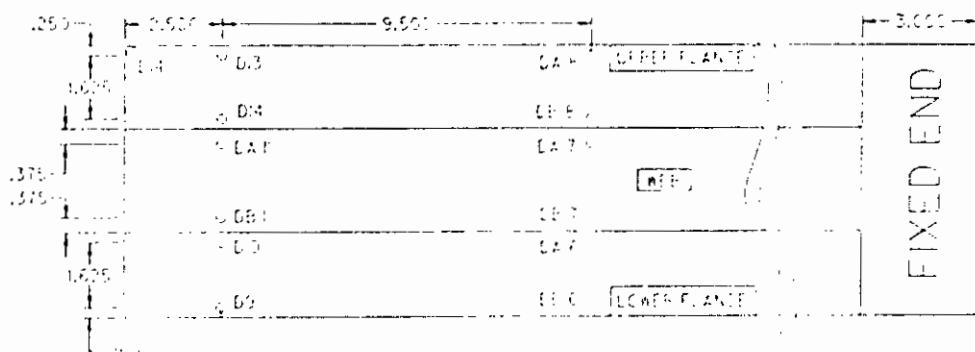


Figure 4. Deflection gauge locations for composite beam load tests.

Another observation was that for beams having the same ply angle, the beams of Form 1 deflected at least twice (100 percent) as much as those of Form 2 when the load was applied near the shear center, and almost 80 percent more when loaded at the geometric center. Additionally, by examining the deflection data at the 0.25-in increments, more interesting facts were revealed. For example, in the case where the cross ply angle is $\pm 75^\circ$, it is noted that the deflection in the web and flanges decrease (for the maximum load case) as one moves toward the calculated shear center located 1.44 in from the geometric center. Yet, as will be discussed later, the deflections approach zero in the web even more by moving further than 1.44 in from the shear center. This serves as an indicator that the true shear center is further than 1.44 in away from the centroid. Therefore, one would opt for the laminate layup of Form 2 in situations in which low deflections are desired.

Two materials systems were used: AS4/3501-6 and IM7/8552. Figures 5 through 8 show deflections measured at the geometric centers and shear centers for a $[0/75]_{12}$ beam of each material. Due to material properties, the IM7/8552 beams deflected more than the AS4/3501-6 beams. Another point of interest from the data gathered is that the further away from the geometric center that the load is applied, the greater the difference in the deflection amounts. The explanation of this could also form the basis for more research work.

AS4/3501-6 [0/75]12

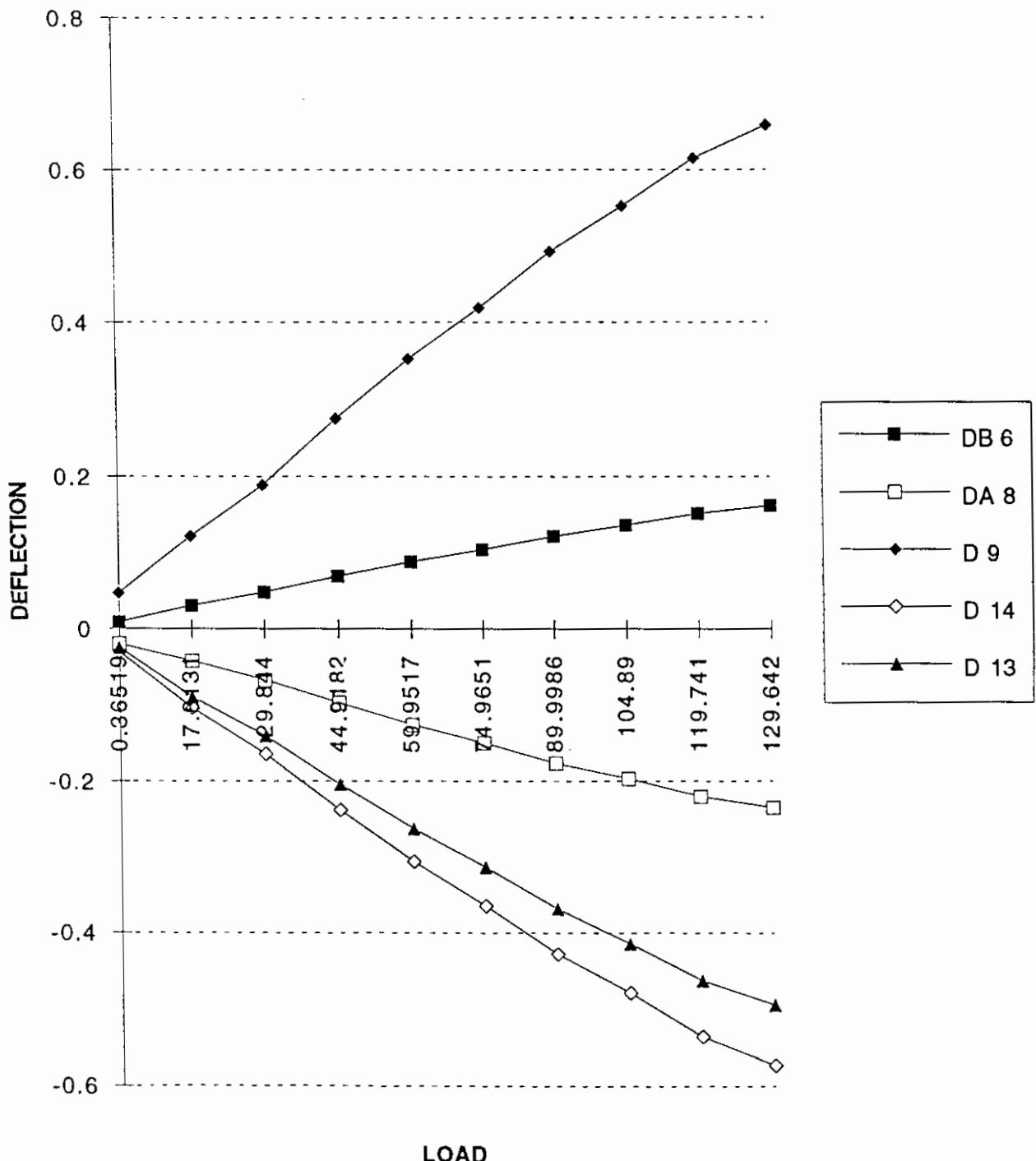


Figure 5. Deflections for C-channel loaded at geometric center (AS4/3501-6).

AS4/3501-6 [0/75]12

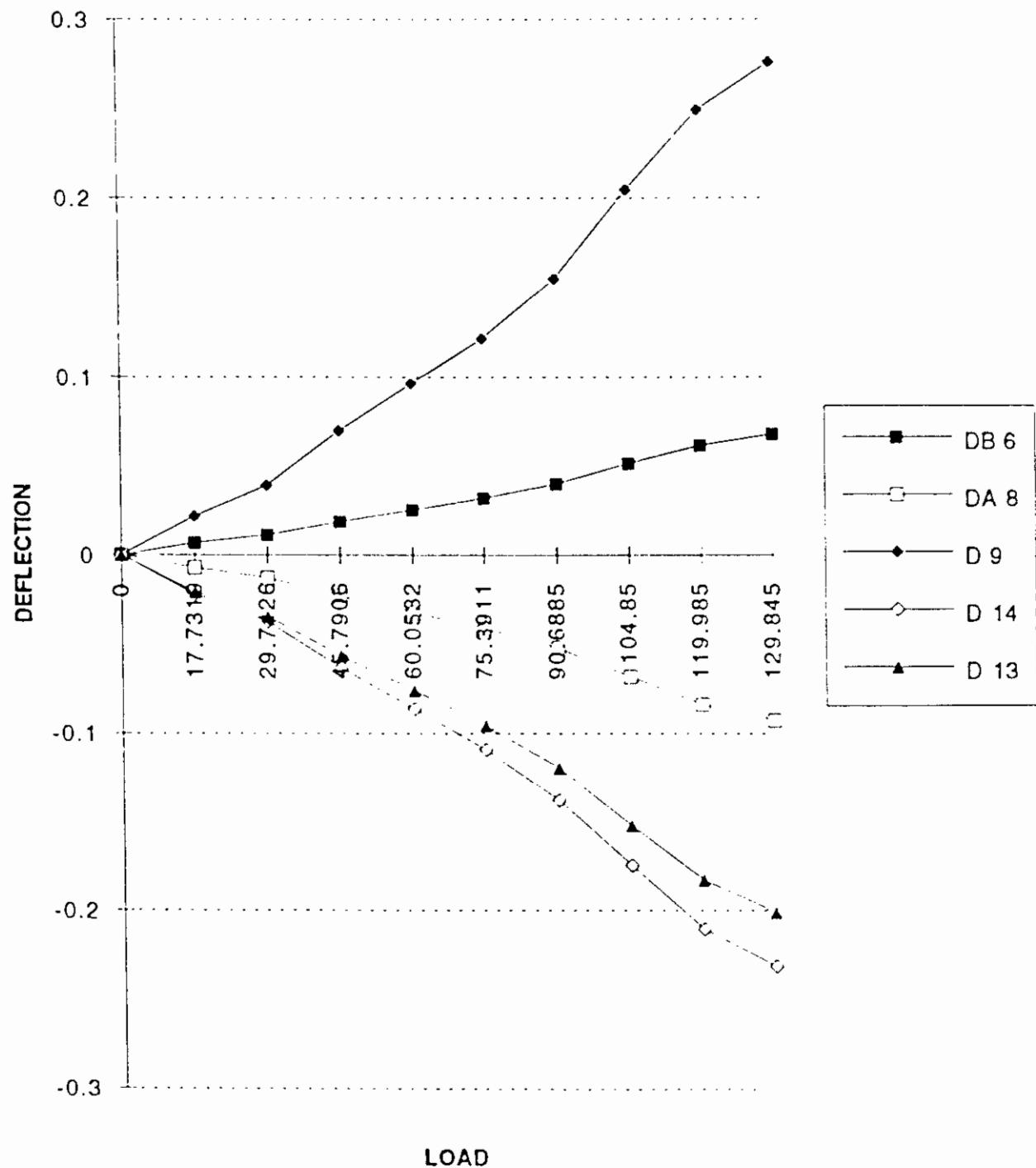


Figure 6. Deflections for C-channel loaded at theoretical shear center (AS4/3501-6).

IM7/8552 [0/75]12

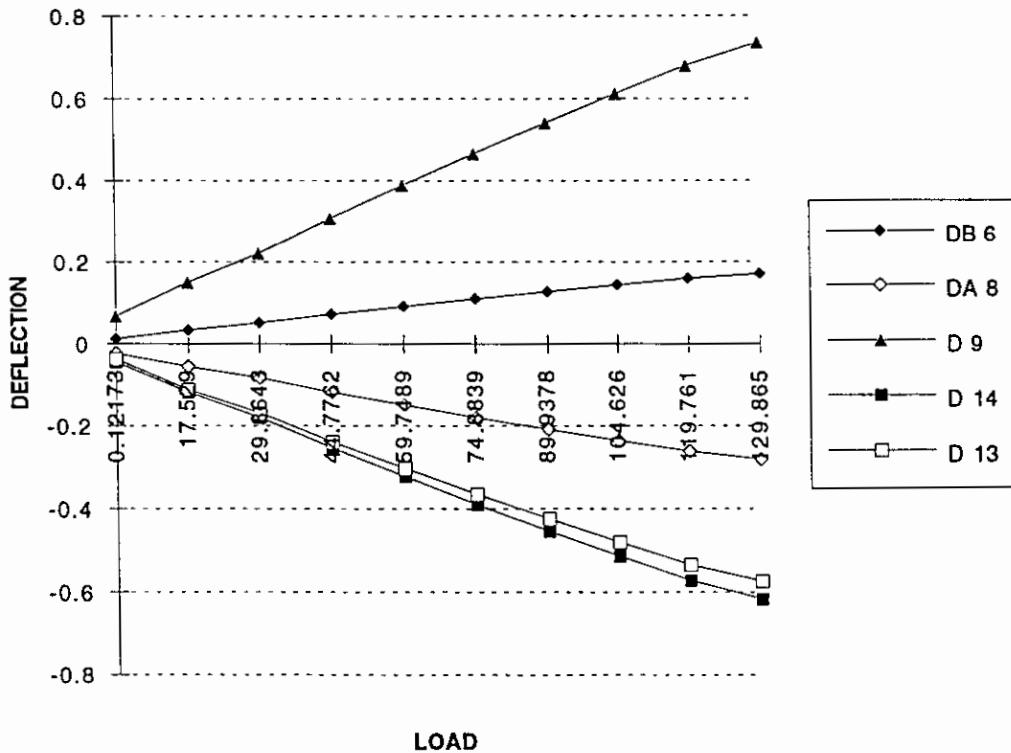


Figure 7. Deflections for C-channel loaded at geometric center (IM7/8552).

IM7/8552 [0/75]12

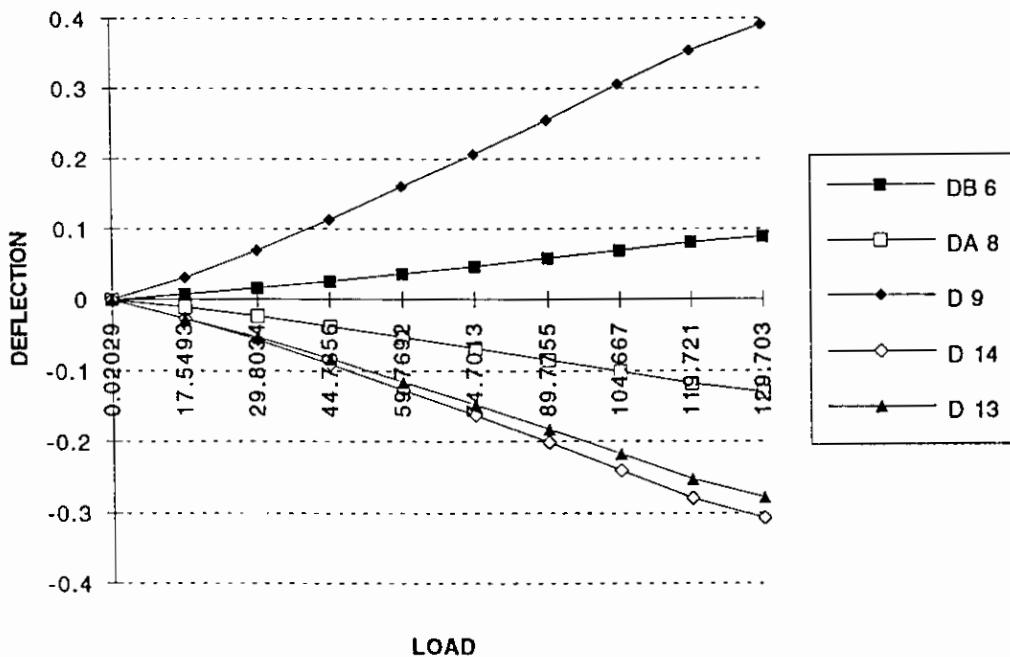


Figure 8. Deflections for C-channel loaded at theoretical shear center (IM7/8552).

EFFECT OF FIBER ORIENTATION OF SHEAR CENTER LOCATION

For this research effort, the theoretical shear center for the given cross section is easily determined by the formula:

$$e = bht/4I .$$

After this shear center is known, one would expect that a transverse load applied through this point would produce only bending, and that the same load applied through the beam's geometric center produces both bending and torsion coupling. Yet, due to the layup of these open-section members (no midplane symmetry) the true shear center does not follow this equation. The research work showed that for some of the laminate layups the true shear center actually lies between the geometric center and theoretical shear center, and could, in some cases, lie beyond the calculated shear center. As the angle increased, the amount of twist observed and measured when the beam was loaded at this "predicted" shear center increased also. As the load application point was shifted toward the center of gravity (c.g.), the trend was for more twisting to occur. However, for most of the cases tested, this increase did not occur until after the load application point had been shifted more than 0.25 in from the shear center in the direction of the c.g. This would indicate that the true shear center is actually between the predicted shear center and a 0.25 in toward the c.g. Yet, for those beams of Form 1 with a cross-ply angle greater than 45°, the "true" shear center was outside the theoretical value, i.e., more than 1.44 in from the geometric center.

Figure 9 shows the angle of twist for various layups as calculated by the formulas³

$$e = d_1/2L \quad \text{and} \quad e = d_2/2L ,$$

$$\phi = d_1/(h/2) \quad \text{and} \quad \phi = d_2/(b/2) ,$$

where

h = height of web

b = width of flange

*d*₁ = web deflection

*d*₂ = flange deflection.

For presentation purposes, beams made with only the 15°, 45°, and 75° cross-ply angles of Form 1 are shown. Measured strains and deflections for the 30° and 60° cases fell between the extremes of the charts shown. Charts 1 through 6 in section II of the appendix contain the strain data as measured for the [(0/15)]₁₂ beam.

Graphically, one can see from figure 10 that the angle of twist does increase with an increase in the cross-ply laminate angle, regardless of the overall layup (Form 1 or Form 2). The range in angle of twist as measured is between -18° and 55° for Form 1, while its range was -25° to 75° for Form 2 beams.

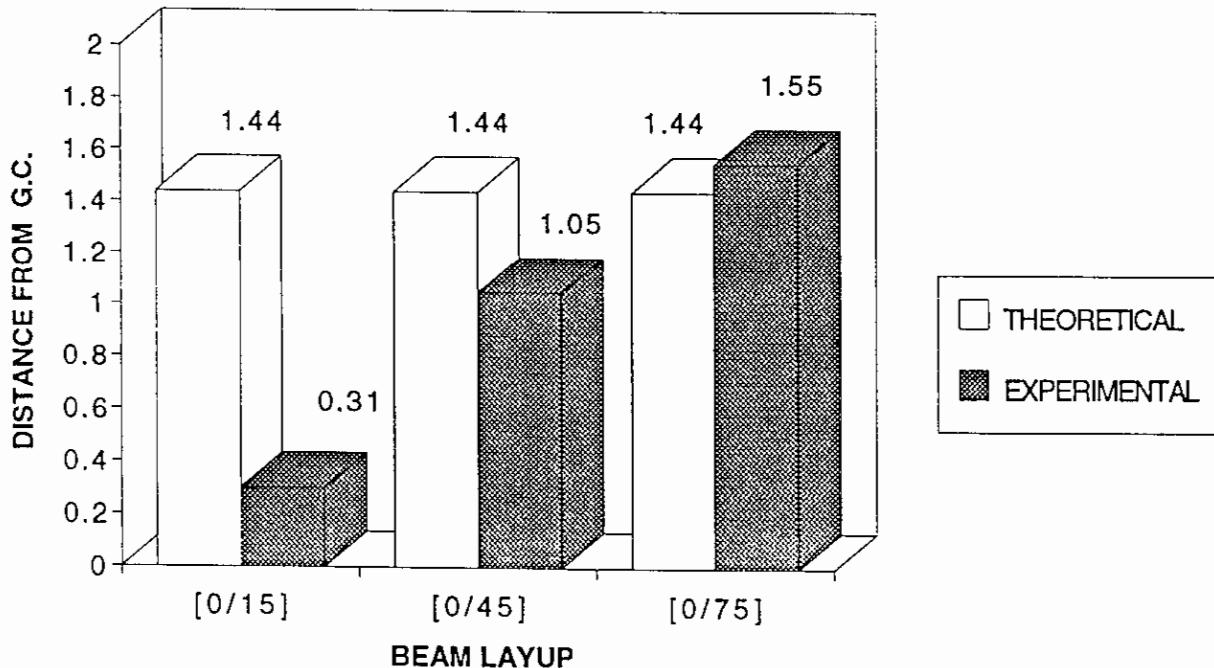


Figure 9. Theoretical versus measured shear center comparison.

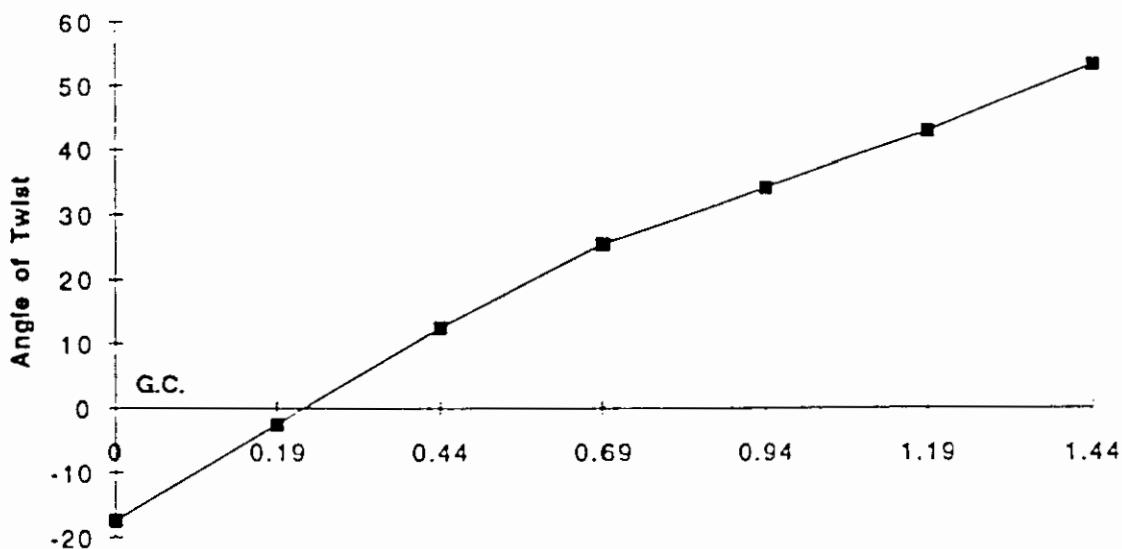
What these equations and formulas do not show, however, is the postcure inward canting of the beams as shown in figure 7. No quantitative assessment of the amount of this cant is presented here. Nevertheless, it should be pointed out that this phenomena occurred in all the beams. Interestingly, the beams containing 45° plies of Form 1 canted outward (i.e., pulled away from the tool after curing) and twisted along the length of the beam by more than 90°. CTE's, as well as interlaminar shear stress and strains during the curing process, would have to be closely examined. The explanation of this occurrence could provide the basis for additional research work.

Test data were also used to gain insight on the location of the beam's "true" shear center (i.e., no twist when transverse load applied) as a function of laminate layup. Due to the beams' geometry, they are expected to twist as well as bend, and all the beams tested experienced this. However, the focus here was to attempt to categorize the amount of twist that could be expected from a given layup. In analyzing the test results, several interesting trends were noticed and are reported here.

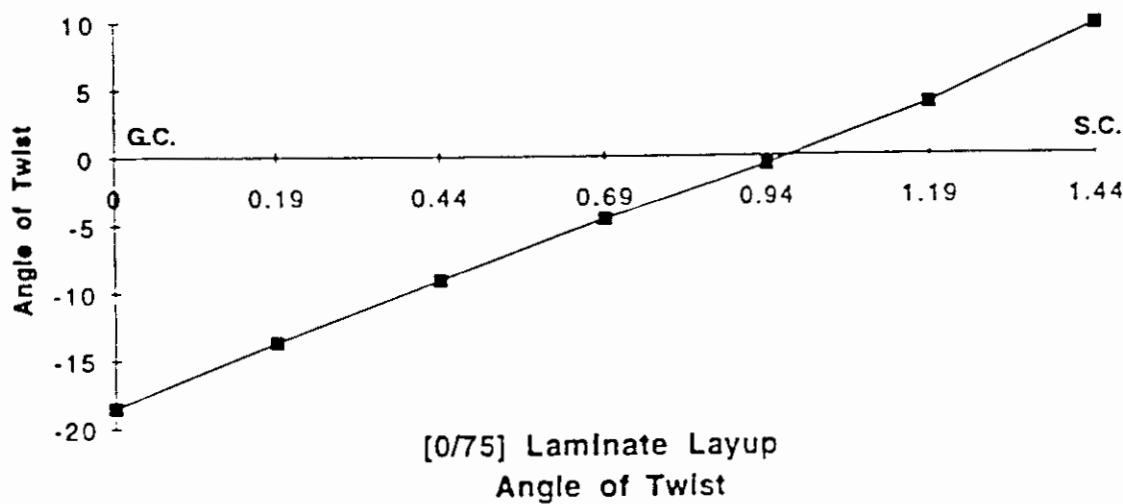
First, the beams of the Form 1 construction had higher degrees of rotation (angles of twist) than those of Form 2 for any given cross-ply lamina. Remembering that the Form 1 beams have cross plies only in the positive direction, whereas the Form 2 beams have both positive and negative plies, it is obvious that, upon completion of the appropriate stiffness matrices, this in itself is not abnormal.

Figure 10 reveals another trend. By calculations using the given geometry of the beams, one can find that the theoretical shear center is located 1.44 in to the left of the geometric center as shown in figure 3. By observing the charts, one can see that none of the plotted beams show an experimental shear center that matches its theoretical value. All the beams were of the same overall geometry and the results shown on the charts were based on a 130-lb transverse load placed on the beams.

[0/15] Laminate Layup
Angle of Twist



[0/45] Laminate Layup
Angle of Twist



[0/75] Laminate Layup
Angle of Twist

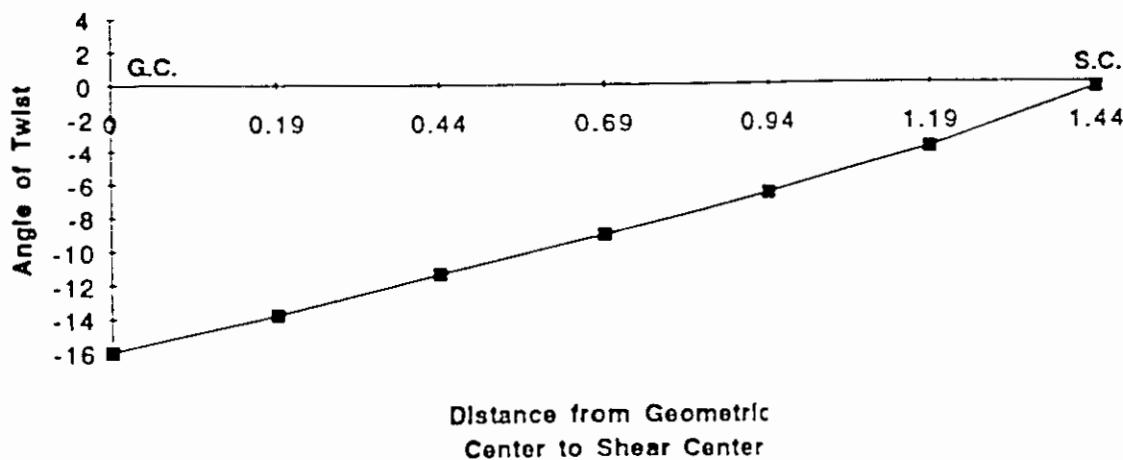


Figure 10. Angle of twist versus load application points for Form 1 beam (AS4/3501-6).

The $[0/75]_{12}$ layup most closely approached the theoretical value and had the highest angle of twist, while the $[0/15]_{12}$ layup had the lowest angle of twist. This tendency, particularly the least twist in the $[0/15]_{12}$ members, was not expected. It would appear intuitively that since the tendency to rotate in these beams would not be offset by the almost horizontal plies of the 15° laminates they would have the higher twist angle. Similarly, these same beams had true shear centers located more than 1 inch from the theoretical value.

All beams had some small amount of twist before loading due to CTE differences between the plies during the cure process. This pretwist was most noticeable in the $[0/45]_{12}$ beams which rotated more than 90° along the length of the beam. Pretwist due to CTE mismatches is not addressed here, but it is worthy of future consideration, as is controlling the various parameters comprising the cure cycle (e.g., cure pressure, tooling materials, cure temperature profiles). This pretwist, however, does not significantly alter the trends reported here since the loads were applied at the calculated theoretical shear center and additional twisting occurred, again verifying the shifting of the "true" shear center. Figures 11(a) and (b) shows the angle of twist for Form 1 beams when Θ is equal to 15, 45, and 75, respectively, and only a minimal load is applied ($P < 5$ lb).

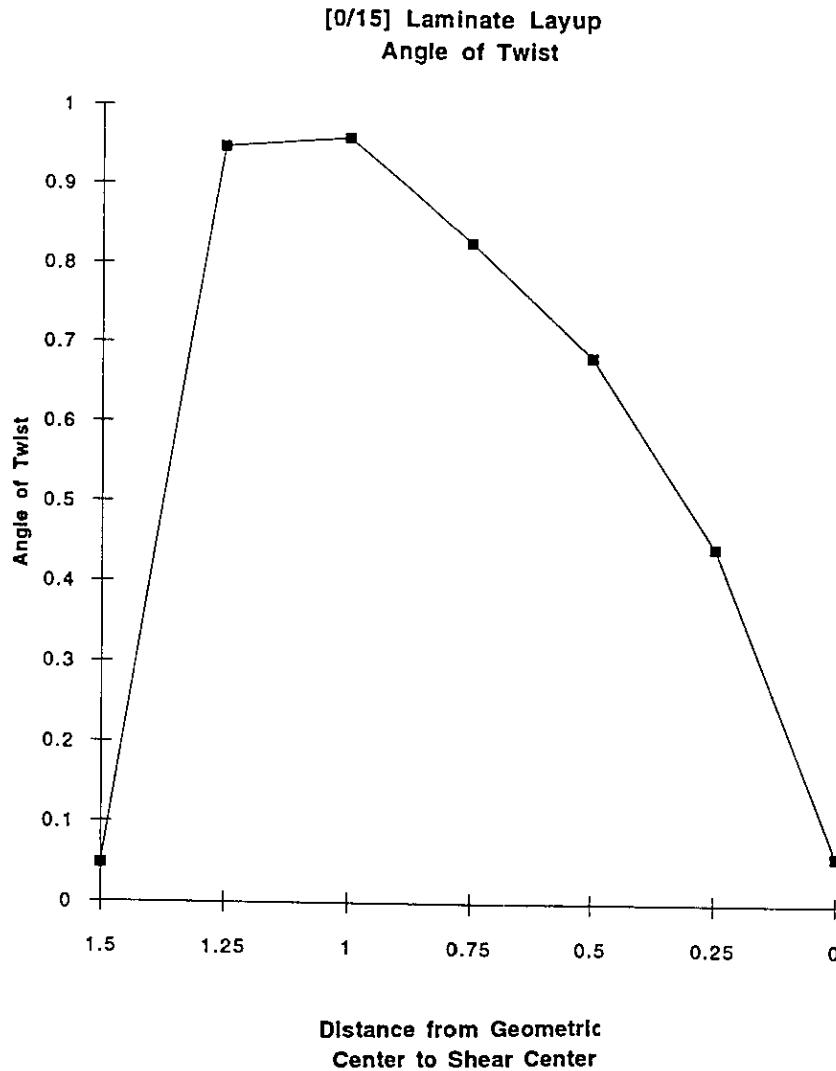
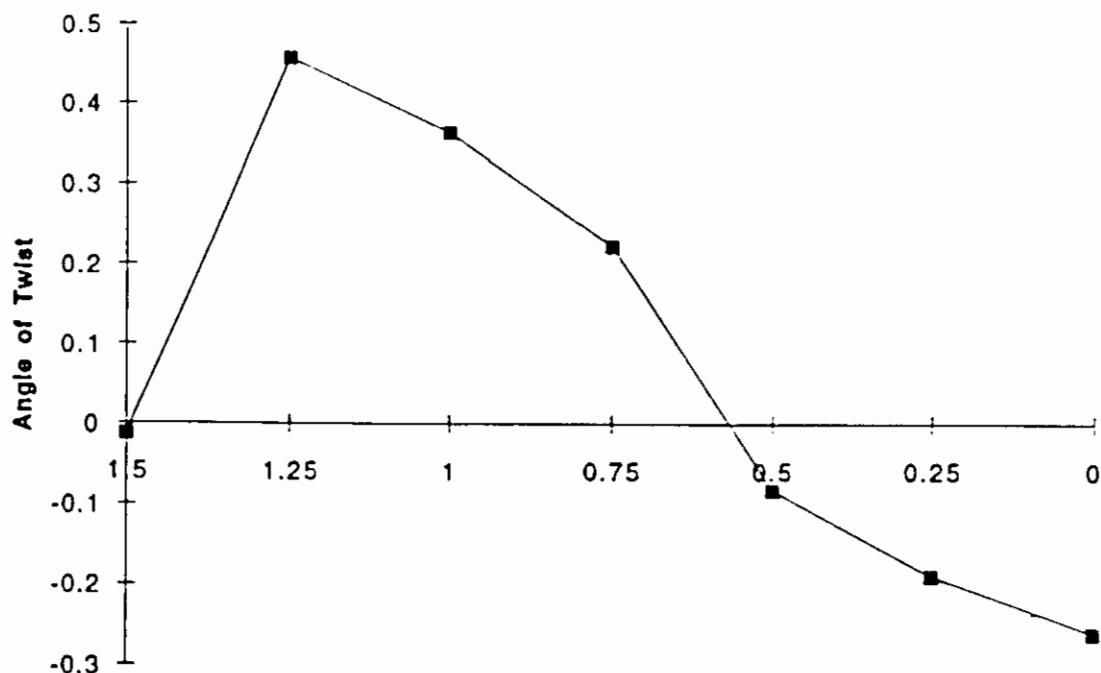
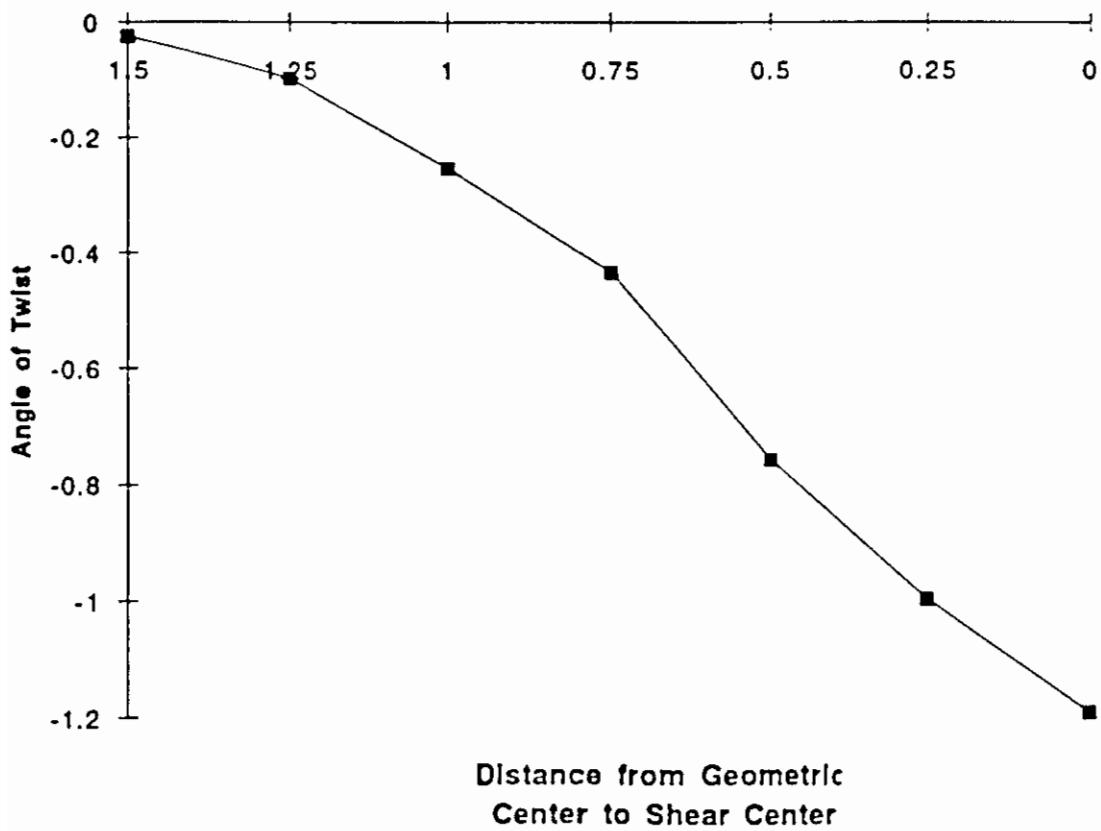


Figure 11(a). Angle of twist versus load application points for Form 1 beam with minimal load applied, $P \leq 5.0$ lb (AS4/3501-6).

[0/45] Laminate Layup
Angle of Twist



[0/75] Laminate Layup
Angle of Twist



**Distance from Geometric
Center to Shear Center**

Figure 11(b). Angle of twist versus load application points for Form 1 beam with minimal load applied, $P \leq 5.0$ lb (AS4/3501-6).

Assuming plate theory, it can be shown that in-plane stress resultants for laminated structures are not only functions of midplane strains but can be functions of curvatures and twists. Additionally, the in-plane forces can cause deformations that can cause curvatures or twisting deformations.⁴ Plate theory does not directly apply to beams as presented in the above cases. As Vinson points out, the above analytical procedures are valid only if no coupling exists, i.e., the D_{16} and D_{26} terms of the stiffness matrix are zero.⁵ The equations were used in this paper only to produce a relative number that would allow numerical comparisons to be made. More complex analysis that considers not only the nonzero cases for D_{16} and D_{26} but also hygrothermal effects would need to be developed to more accurately assess the true values for the amount of twist produced due to laminate angle orientation.

Even though plate theory may not be directly applicable to beam theory, one can use it to gather trend data. This is an attempt to categorize the behavior of the laminates rather than assign a quantitative value for the angle of twist. Regardless of the number assigned to this angle, the observed trends would still be valid. The analysis method used to arrive at this value does not influence the direction or importance of the trend.

CONCLUSIONS

The magnitude of information gathered through tests performed on open-section unsymmetric beams was tremendous. All data gathered were not discussed in this paper; however, subsequent writings will explore other areas worthy of further investigation in an attempt to more accurately categorize the behavior of such beams. Major findings reported here deal with the primary objective regarding shear center location and manipulation, as well as angle of twist and warpage in these structural members.

One important revelation is that the angles of twist and associated warpage in open-section beams is higher for antisymmetric beams of the $[0/\phi]_m$ laminate layup. Some amount of bending/stretching is expected in any antisymmetric layup due to the fact that the components of the $[B]$ coupling matrix, which relates stress couples to midsurface planes and stress resultants to curvatures, are not all zero if the structure is not symmetric about its midplane. Likewise, unless the D_{16} and D_{26} terms of the stiffness matrix $[D]$, which relates the stress couples to curvatures, are zero, bending/twisting will occur.⁶ No attempt is made to measure the magnitude of hygrothermal effects on twist in the beam, even though there could be as many as three different CTE's depending on cross-ply angle and orientation, all which play a role in the warping of the beams.⁷

The major fact borne out here is that laminate layup does control shear center location. As evidenced by charts presented earlier, the same geometric shape can yield "true" shear centers that differ by more than an inch depending on laminate layup. The location of the "true" shear center for the type beam investigated here depends on several factors. Cross-ply angle sequence is important (Form 1 or Form 2) in that other unsymmetrical layups using the same angle would yield different shear center locations. Yet not to be overlooked are factors such as cure cycle and laminate materials.

REFERENCES

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2. Nemeth, M.P.: "Buckling Behavior of Long Symmetrically Laminated Plates Subjected to Combined Loadings," LaRC, NASA Technical Paper 3195.
3. Vinson, J.R., and Serakowski, R.L.: "The Behavior of Structures Composed of Composite Materials," Martinus Nijhoff Publisher, 1986, pp. 101-103.
4. Ibid., p. 54.
5. Ibid., p. 82.
6. Ibid., p. 56.
7. Ibid. p. 41.

APPENDIX

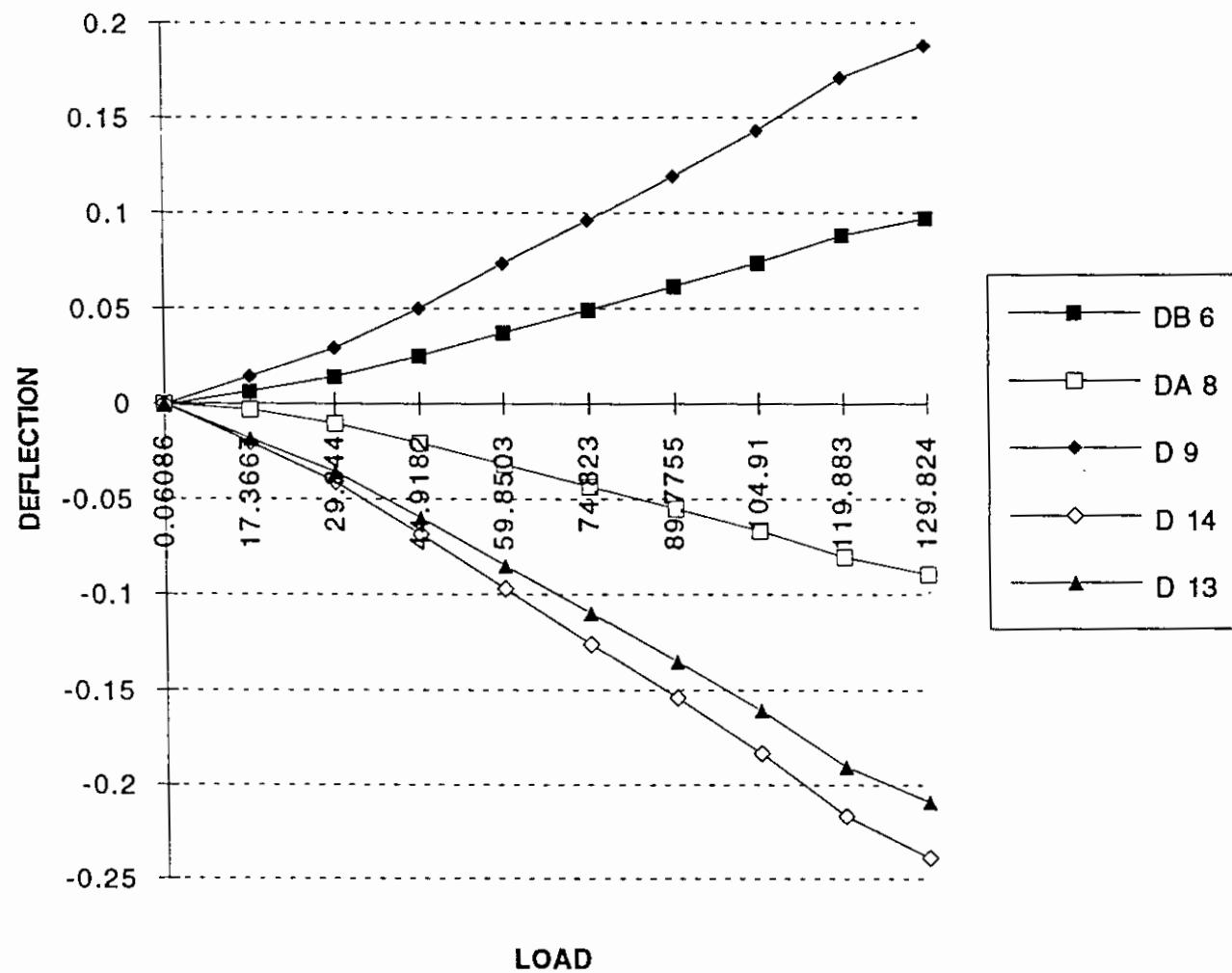
Contents

- I. Deflection Graphs and Data**
- II. Test Strain Data**
- III. Manufacturing and Testing Photographs**

SECTION I
Deflection Graphs and Data

IM7/8552 1.5 IN GC GRAPH

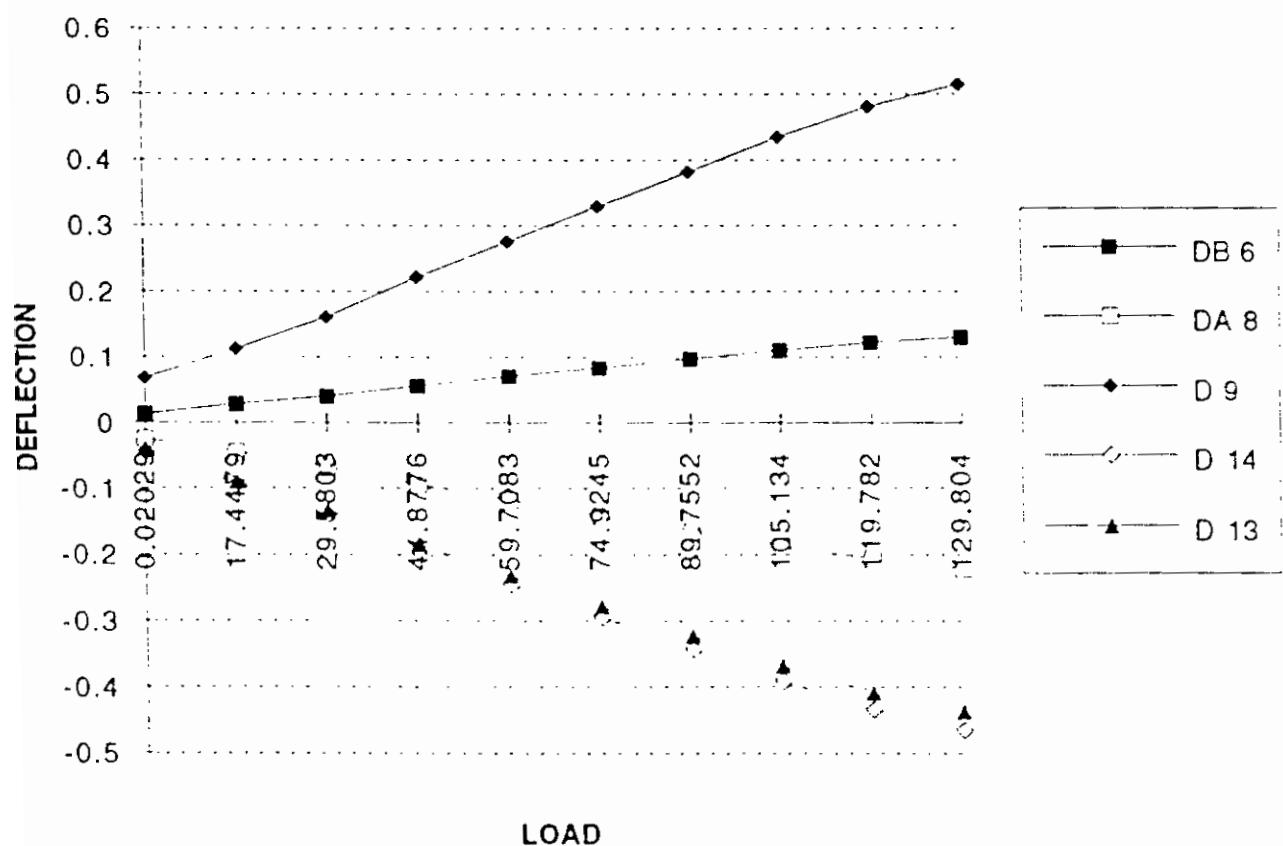
IM7/8552 [0/15]12



Graph 1

R13 GRAPH 0.0INCG

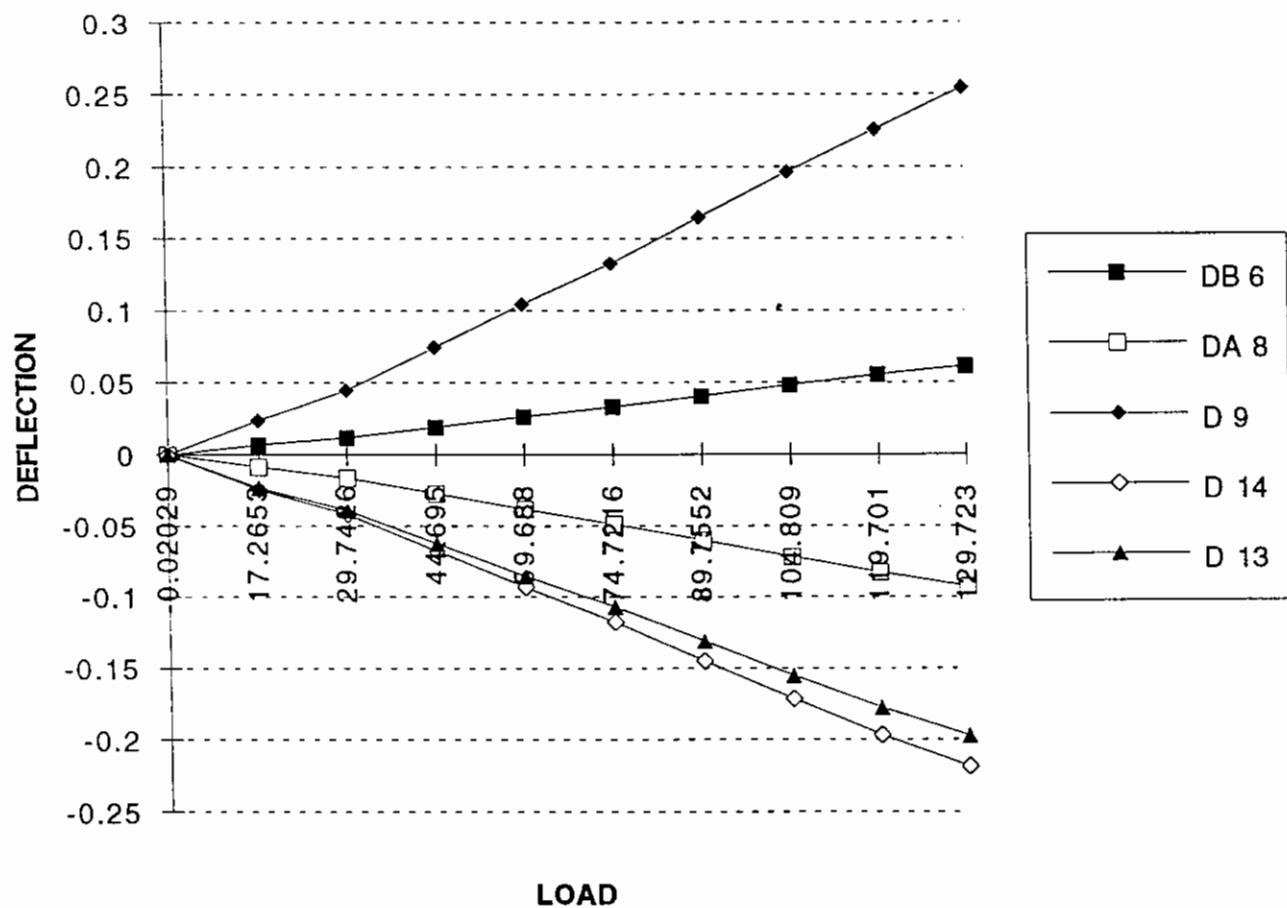
AS4/3501-6 [(0/30)6,(0/-30)6]



Graph 2

R13 GRAPH 1.5IN

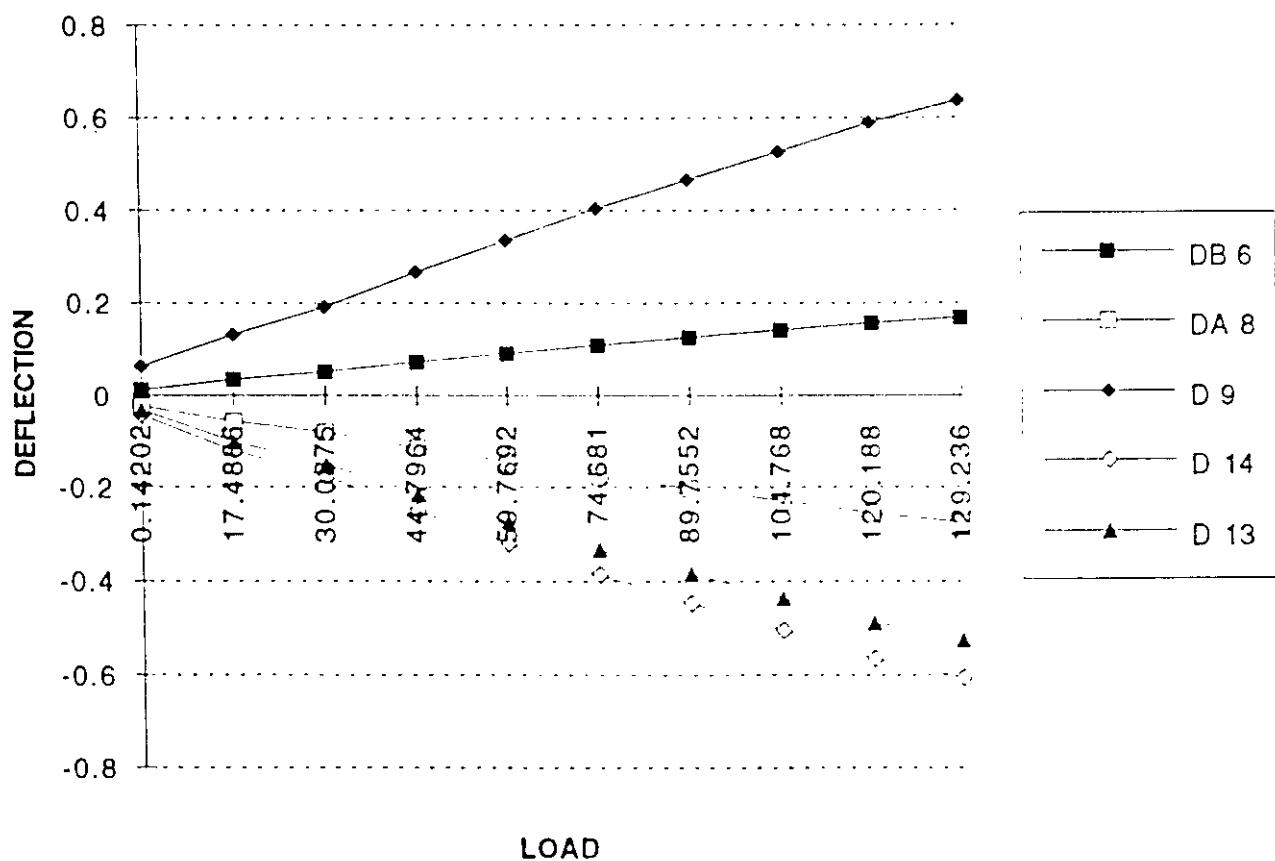
AS4/3501-6 [(0/30)6,(0/-30)6]



Graph 3

R11 GRAPH 0.0IN

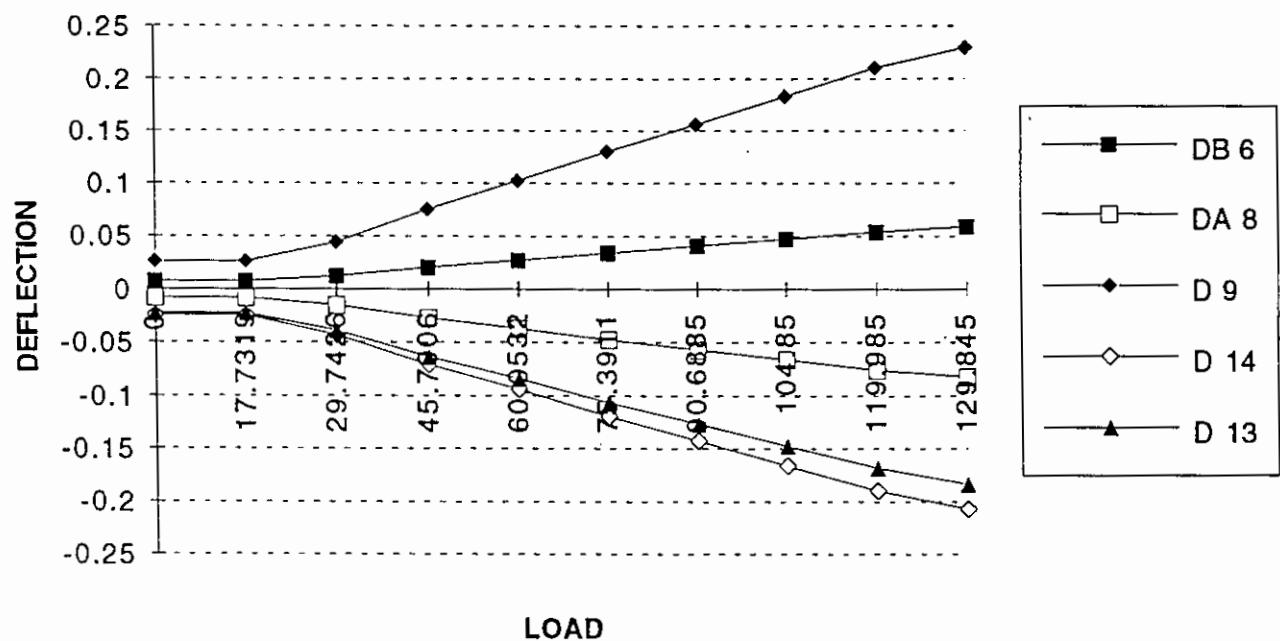
IM7/8552 [(0/45)6,(0/-45)6]



Graph 4

AS4/3501-6 1.5 IN GC GRAPH

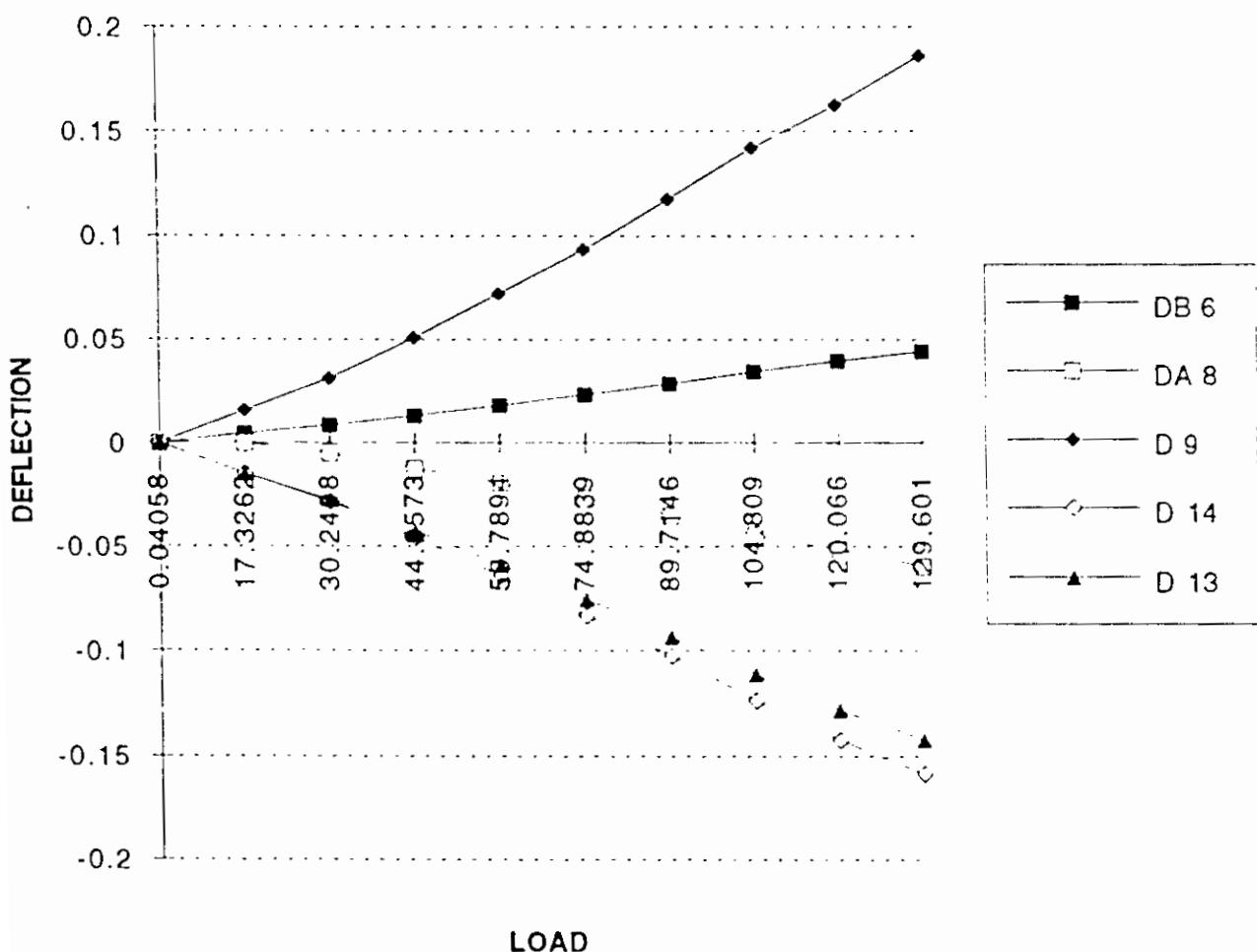
AS4/3501-6 [0/60]12



Graph 5

R15 GRAPH 1.5IN

IM7/8552 [(0/75)6,(0/-75)6]



Graph 6

DEFLECTIONS

| PT CDDF COMPBEAM S/N 20 RUN 19 | | " L 1" | " DB 8" | " DB 7" |
|--------------------------------|-----------|-----------|----------|----------|
| | | " LBS " | " INCH " | " INCH " |
| SCAN"DELTA MINUTES | | | | |
| 10 | 0.075000 | 0 | -0.00003 | -0.00029 |
| 17 | 0.816650 | 17.73192 | -0.00438 | 0.00036 |
| 25 | 1.366633 | 29.74255 | -0.00987 | 0.0004 |
| 33 | 1.816633 | 45.79056 | -0.01885 | 0.00129 |
| 41 | 2.149950 | 60.05319 | -0.02773 | 0.00212 |
| 49 | 2.574950 | 75.39111 | -0.03662 | 0.00287 |
| 57 | 3.116600 | 90.68845 | -0.04752 | 0.00423 |
| 65 | 3.591583 | 104.84964 | -0.06319 | 0.00425 |
| 73 | 4.108250 | 119.98465 | -0.07913 | 0.00521 |
| 81 | 4.524900 | 129.84473 | -0.0887 | 0.00526 |
| 89 | 5.033217 | 0.30432 | -0.01173 | -0.00067 |
| 97 | 5.891533 | 17.79279 | -0.01073 | 0.001 |
| 105 | 6.341533 | 30.0063 | -0.0179 | 0.00218 |
| 113 | 6.733183 | 45.24277 | -0.02924 | 0.00377 |
| 121 | 7.258167 | 59.87059 | -0.04203 | 0.00552 |
| 129 | 7.624833 | 76.02005 | -0.05608 | 0.00776 |
| 137 | 8.166483 | 90.20151 | -0.06726 | 0.00881 |
| 145 | 8.616467 | 105.01193 | -0.08014 | 0.00987 |
| 153 | 9.091467 | 120.26868 | -0.09277 | 0.01083 |
| 161 | 9.483117 | 131.2243 | -0.1036 | 0.01133 |
| 169 | 10.049783 | 0.36519 | -0.013 | -0.00065 |
| 177 | 10.516433 | 17.83336 | -0.01542 | 0.00246 |
| 185 | 11.008083 | 30.14832 | -0.02274 | 0.00406 |
| 194 | 11.449750 | 44.99931 | -0.03615 | 0.00654 |
| 201 | 12.033067 | 60.19521 | -0.04989 | 0.00894 |
| 210 | 12.499717 | 75.26938 | -0.06231 | 0.01102 |
| 226 | 13.274700 | 104.68732 | -0.08884 | 0.01536 |
| 235 | 13.716367 | 120.83676 | -0.10312 | 0.01685 |
| 241 | 14.316350 | 130.65625 | -0.11419 | 0.01751 |
| 250 | 15.049667 | 0.3449 | -0.01225 | -0.00115 |
| 257 | 15.549650 | 17.79279 | -0.01579 | 0.0033 |
| 265 | 16.024650 | 30.10774 | -0.02581 | 0.00568 |
| 273 | 16.441300 | 45.03989 | -0.04019 | 0.00939 |
| 282 | 16.899633 | 60.49953 | -0.0544 | 0.01218 |
| 290 | 17.507950 | 74.98535 | -0.06911 | 0.01525 |
| 298 | 17.949600 | 89.75519 | -0.08329 | 0.01814 |
| 305 | 18.357933 | 104.76846 | -0.0974 | 0.02026 |
| 314 | 18.824583 | 119.98465 | -0.1124 | 0.02229 |
| 321 | 19.141250 | 130.16931 | -0.12092 | 0.02262 |
| 330 | 20.066217 | 0.30432 | -0.01279 | -0.00089 |
| 331 | 20.524550 | 17.73192 | -0.01868 | 0.00485 |
| 346 | 21.041200 | 29.88457 | -0.02853 | 0.00717 |

DEFLECTIONS

| | | | | | |
|-----|-----------|--|-----------|----------|----------|
| 354 | 21.591200 | | 44.95874 | -0.04423 | 0.01114 |
| 362 | 22.074517 | | 59.83002 | -0.05784 | 0.01467 |
| 375 | 22.557833 | | 74.98535 | -0.07449 | 0.01823 |
| 385 | 23.066167 | | 89.91748 | -0.08921 | 0.02145 |
| 393 | 23.624483 | | 105.05252 | -0.10437 | 0.02332 |
| 402 | 24.066133 | | 119.74121 | -0.11879 | 0.02575 |
| 410 | 24.507800 | | 129.92584 | -0.12942 | 0.02698 |
| 418 | 25.649433 | | 0.36519 | -0.01371 | -0.00098 |
| 434 | 26.507750 | | 29.80342 | -0.03241 | 0.00968 |
| 442 | 27.082733 | | 44.8573 | -0.0484 | 0.01453 |
| 451 | 27.724400 | | 59.87059 | -0.06498 | 0.01849 |
| 458 | 28.366050 | | 74.92447 | -0.08143 | 0.02207 |
| 467 | 28.974367 | | 90.20151 | -0.09703 | 0.02549 |
| 475 | 29.582683 | | 105.09308 | -0.11432 | 0.02889 |
| 482 | 30.216000 | | 119.9035 | -0.13006 | 0.03171 |
| 490 | 30.741000 | | 129.96643 | -0.1412 | 0.03495 |
| 498 | 31.432650 | | 0.36519 | -0.01459 | -0.00074 |
| 505 | 32.090967 | | 17.81307 | -0.02229 | 0.00681 |
| 513 | 32.607617 | | 29.84399 | -0.0364 | 0.0119 |
| 522 | 33.374267 | | 44.91815 | -0.05346 | 0.01703 |
| 529 | 33.957600 | | 59.95174 | -0.07144 | 0.02253 |
| 537 | 34.532583 | | 74.96506 | -0.08766 | 0.02635 |
| 546 | 35.082567 | | 89.99863 | -0.10573 | 0.03084 |
| 554 | 35.590883 | | 104.8902 | -0.12126 | 0.03331 |
| 562 | 36.140883 | | 119.74121 | -0.13889 | 0.03703 |
| 570 | 36.882517 | | 129.64185 | -0.15148 | 0.03842 |

Chart 2

DEFLECTIONS

| " DB 6" | " DB 11" | " DA 8" | " DA 7" | " DA 6" | " DA 11" |
|----------|----------|----------|----------|----------|----------|
| " INCH " |
| -0.00008 | -0.00028 | 0.00002 | -0.00007 | -0.00013 | -0.00011 |
| 0.00689 | 0.00122 | -0.00676 | -0.00012 | 0.00738 | -0.00185 |
| 0.01142 | 0.00235 | -0.01257 | -0.0001 | 0.01311 | -0.00243 |
| 0.01897 | 0.00485 | -0.02238 | -0.00013 | 0.0219 | -0.00366 |
| 0.02542 | 0.00687 | -0.03193 | -0.0001 | 0.03001 | -0.00408 |
| 0.032 | 0.00922 | -0.04071 | -0.0001 | 0.03854 | -0.00299 |
| 0.04023 | 0.01242 | -0.05194 | 0.00023 | 0.04998 | -0.00101 |
| 0.05155 | 0.01261 | -0.06814 | 0.00021 | 0.06578 | -0.00221 |
| 0.06194 | 0.01492 | -0.08356 | 0.00074 | 0.08113 | -0.00144 |
| 0.06822 | 0.01539 | -0.09245 | 0.00178 | 0.09037 | -0.00052 |
| 0.00293 | -0.00024 | -0.00846 | 0.00251 | 0.00613 | 0.00183 |
| 0.01259 | 0.00471 | -0.01544 | -0.00086 | 0.01282 | -0.00677 |
| 0.02014 | 0.00776 | -0.02517 | -0.00291 | 0.02049 | -0.01112 |
| 0.03057 | 0.01134 | -0.03949 | -0.00499 | 0.03173 | -0.01602 |
| 0.04172 | 0.01609 | -0.05513 | -0.00694 | 0.04436 | -0.01989 |
| 0.05328 | 0.02047 | -0.07346 | -0.0091 | 0.05826 | -0.02378 |
| 0.06227 | 0.02348 | -0.08434 | -0.00918 | 0.06948 | -0.02494 |
| 0.07276 | 0.0264 | -0.09908 | -0.00912 | 0.08208 | -0.02735 |
| 0.0822 | 0.02885 | -0.11244 | -0.00915 | 0.09475 | -0.0283 |
| 0.08972 | 0.03035 | -0.1235 | -0.00913 | 0.1048 | -0.02879 |
| 0.00467 | -0.00019 | -0.01266 | 0.00094 | 0.00769 | -0.00075 |
| 0.01742 | 0.00748 | -0.02275 | -0.00477 | 0.01684 | -0.01262 |
| 0.02667 | 0.01186 | -0.03478 | -0.0077 | 0.02543 | -0.01983 |
| 0.03942 | 0.0175 | -0.05317 | -0.01115 | 0.03819 | -0.02789 |
| 0.0523 | 0.02334 | -0.07193 | -0.01473 | 0.05214 | -0.03498 |
| 0.06431 | 0.02847 | -0.08856 | -0.01703 | 0.06474 | -0.04068 |
| 0.08708 | 0.0375 | -0.12059 | -0.02045 | 0.09039 | -0.04911 |
| 0.09849 | 0.04099 | -0.13644 | -0.02094 | 0.10423 | -0.05255 |
| 0.10644 | 0.04282 | -0.14795 | -0.02152 | 0.11454 | -0.05421 |
| 0.00472 | -0.00118 | -0.00739 | 0.00041 | 0.00725 | -0.00207 |
| 0.02034 | 0.00951 | -0.02621 | -0.00742 | 0.018 | -0.01834 |
| 0.03194 | 0.01501 | -0.0426 | -0.01212 | 0.02843 | -0.02845 |
| 0.04688 | 0.02268 | -0.0639 | -0.01726 | 0.04202 | -0.03935 |
| 0.06128 | 0.02969 | -0.08454 | -0.02155 | 0.05667 | -0.04858 |
| 0.0753 | 0.03703 | -0.10441 | -0.02565 | 0.0715 | -0.05701 |
| 0.08813 | 0.04273 | -0.12268 | -0.02879 | 0.08517 | -0.06404 |
| 0.09994 | 0.04649 | -0.14017 | -0.03011 | 0.09889 | -0.06905 |
| 0.11233 | 0.05162 | -0.15803 | -0.03212 | 0.11291 | -0.0747 |
| 0.1192 | 0.05294 | -0.16737 | -0.0326 | 0.12128 | -0.07713 |
| 0.00636 | -0.00061 | -0.01483 | -0.00319 | 0.00732 | -0.00628 |
| 0.02466 | 0.01266 | -0.03508 | -0.01277 | 0.021 | -0.02544 |
| 0.03681 | 0.01859 | -0.04956 | -0.01652 | 0.03074 | -0.03668 |

Chart 3

DEFLECTIONS

| | | | | | |
|---------|----------|----------|----------|---------|----------|
| 0.0534 | 0.02828 | -0.07406 | -0.02301 | 0.04627 | -0.05064 |
| 0.06856 | 0.036 | -0.09511 | -0.02776 | 0.06035 | -0.06191 |
| 0.08386 | 0.04339 | -0.1169 | -0.03252 | 0.07567 | -0.07231 |
| 0.09771 | 0.04979 | -0.13726 | -0.03662 | 0.09026 | -0.08143 |
| 0.11087 | 0.05435 | -0.15655 | -0.03936 | 0.10458 | -0.08909 |
| 0.12367 | 0.05915 | -0.17523 | -0.04177 | 0.11877 | -0.09608 |
| 0.1322 | 0.06174 | -0.18744 | -0.0436 | 0.12886 | -0.10008 |
| 0.0077 | -0.00042 | -0.01831 | -0.00466 | 0.00925 | -0.00789 |
| 0.04277 | 0.02292 | -0.05831 | -0.02104 | 0.03473 | -0.04557 |
| 0.06108 | 0.03369 | -0.08503 | -0.02899 | 0.05048 | -0.06288 |
| 0.07845 | 0.04273 | -0.10936 | -0.03582 | 0.06626 | -0.07735 |
| 0.09453 | 0.05106 | -0.13374 | -0.04128 | 0.08195 | -0.09089 |
| 0.10966 | 0.05807 | -0.15614 | -0.04623 | 0.09757 | -0.10199 |
| 0.12502 | 0.0647 | -0.17883 | -0.05067 | 0.11326 | -0.11321 |
| 0.13873 | 0.06955 | -0.19874 | -0.05447 | 0.12833 | -0.1214 |
| 0.1478 | 0.07346 | -0.21218 | -0.05794 | 0.13882 | -0.12655 |
| 0.00887 | -0.00024 | -0.01974 | -0.00745 | 0.01071 | -0.00983 |
| 0.03099 | 0.01671 | -0.04203 | -0.01713 | 0.02415 | -0.03679 |
| 0.0482 | 0.02786 | -0.06691 | -0.02613 | 0.03777 | -0.05518 |
| 0.06921 | 0.04009 | -0.09724 | -0.03566 | 0.05498 | -0.07661 |
| 0.08806 | 0.05073 | -0.1255 | -0.04435 | 0.07197 | -0.0941 |
| 0.10458 | 0.05882 | -0.14938 | -0.05059 | 0.08733 | -0.10939 |
| 0.12177 | 0.06743 | -0.17597 | -0.05695 | 0.10409 | -0.12397 |
| 0.13643 | 0.07284 | -0.19669 | -0.06141 | 0.11908 | -0.13583 |
| 0.15214 | 0.08 | -0.22037 | -0.06686 | 0.13516 | -0.14749 |
| 0.1625 | 0.08263 | -0.2352 | -0.06897 | 0.14649 | -0.15392 |

Chart 4

DEFLECTIONS

| " D 9" | " D 14" | " D 13" | " D 12" | " D 10" |
|----------|----------|----------|----------|----------|
| " INCH " |
| 0 | 0.00019 | -0.00005 | -0.0001 | -0.00005 |
| 0.02193 | -0.02177 | -0.02025 | -0.01786 | 0.01736 |
| 0.03925 | -0.03713 | -0.03398 | -0.03048 | 0.0297 |
| 0.06991 | -0.06337 | -0.05634 | -0.04965 | 0.04894 |
| 0.0965 | -0.0863 | -0.07592 | -0.06687 | 0.06615 |
| 0.12187 | -0.10924 | -0.09535 | -0.08512 | 0.08411 |
| 0.15467 | -0.13743 | -0.11916 | -0.10826 | 0.10769 |
| 0.20454 | -0.17455 | -0.15203 | -0.13966 | 0.13842 |
| 0.24951 | -0.21051 | -0.18245 | -0.16916 | 0.16842 |
| 0.2761 | -0.23112 | -0.20069 | -0.18804 | 0.18711 |
| 0.01683 | -0.0105 | -0.00926 | -0.01037 | 0.0113 |
| 0.0441 | -0.03985 | -0.03671 | -0.02891 | 0.02994 |
| 0.07564 | -0.06628 | -0.05922 | -0.04598 | 0.04563 |
| 0.11741 | -0.10205 | -0.08964 | -0.07025 | 0.06951 |
| 0.16219 | -0.14015 | -0.12208 | -0.09671 | 0.09595 |
| 0.21594 | -0.18077 | -0.15587 | -0.12469 | 0.12353 |
| 0.24825 | -0.21012 | -0.18245 | -0.14842 | 0.14706 |
| 0.29114 | -0.24589 | -0.21345 | -0.17464 | 0.17315 |
| 0.32937 | -0.27835 | -0.24186 | -0.20066 | 0.19856 |
| 0.36158 | -0.30362 | -0.26384 | -0.22082 | 0.21859 |
| 0.02634 | -0.01711 | -0.01473 | -0.01375 | 0.01475 |
| 0.06472 | -0.05695 | -0.0512 | -0.03713 | 0.03779 |
| 0.10242 | -0.08883 | -0.07856 | -0.05645 | 0.05668 |
| 0.15292 | -0.13237 | -0.11556 | -0.08385 | 0.08391 |
| 0.20794 | -0.17689 | -0.15385 | -0.11315 | 0.11277 |
| 0.25601 | -0.21829 | -0.18888 | -0.14093 | 0.13951 |
| 0.34955 | -0.29682 | -0.25664 | -0.19558 | 0.19274 |
| 0.39564 | -0.33511 | -0.29014 | -0.22454 | 0.22086 |
| 0.42786 | -0.36193 | -0.31384 | -0.24538 | 0.24148 |
| 0.01596 | -0.01283 | -0.0132 | -0.01247 | 0.0147 |
| 0.07748 | -0.06784 | -0.0596 | -0.0407 | 0.04168 |
| 0.12318 | -0.10691 | -0.09348 | -0.06364 | 0.06379 |
| 0.1831 | -0.15822 | -0.13729 | -0.09432 | 0.09348 |
| 0.24262 | -0.20818 | -0.18 | -0.12577 | 0.12367 |
| 0.29958 | -0.25639 | -0.22142 | -0.15703 | 0.15406 |
| 0.35169 | -0.30051 | -0.25962 | -0.18658 | 0.18223 |
| 0.40224 | -0.3425 | -0.29522 | -0.21534 | 0.20981 |
| 0.45405 | -0.38565 | -0.33285 | -0.24577 | 0.23871 |
| 0.48142 | -0.40781 | -0.35329 | -0.26348 | 0.25608 |
| 0.03508 | -0.02294 | -0.01795 | -0.01355 | 0.01529 |
| 0.10251 | -0.08339 | -0.0716 | -0.04603 | 0.04622 |
| 0.1439 | -0.12363 | -0.10711 | -0.06966 | 0.06941 |

DEFLECTIONS

| | | | | |
|---------|----------|----------|----------|---------|
| 0.21143 | -0.18174 | -0.15702 | -0.10366 | 0.10197 |
| 0.27271 | -0.23423 | -0.20169 | -0.13541 | 0.13265 |
| 0.33374 | -0.28749 | -0.2481 | -0.16887 | 0.16398 |
| 0.39244 | -0.3355 | -0.28975 | -0.20086 | 0.19353 |
| 0.44707 | -0.38176 | -0.33016 | -0.23217 | 0.22332 |
| 0.4983 | -0.42725 | -0.36893 | -0.26279 | 0.25253 |
| 0.53478 | -0.4564 | -0.39466 | -0.28432 | 0.27325 |
| 0.04303 | -0.02682 | -0.02159 | -0.01649 | 0.02146 |
| 0.16815 | -0.14481 | -0.12477 | -0.07861 | 0.0815 |
| 0.24253 | -0.20857 | -0.17957 | -0.11491 | 0.11534 |
| 0.31579 | -0.26902 | -0.23121 | -0.15042 | 0.14829 |
| 0.38245 | -0.32695 | -0.2815 | -0.18579 | 0.18085 |
| 0.44319 | -0.38021 | -0.32805 | -0.21994 | 0.21267 |
| 0.50587 | -0.43502 | -0.37565 | -0.25506 | 0.24463 |
| 0.56137 | -0.48323 | -0.41759 | -0.28803 | 0.27512 |
| 0.59999 | -0.51549 | -0.44562 | -0.31112 | 0.29633 |
| 0.04691 | -0.03013 | -0.02438 | -0.01903 | 0.02314 |
| 0.12163 | -0.10438 | -0.08955 | -0.05523 | 0.05772 |
| 0.18916 | -0.16406 | -0.14085 | -0.08654 | 0.08761 |
| 0.27474 | -0.2385 | -0.20443 | -0.12699 | 0.12422 |
| 0.35256 | -0.30615 | -0.26288 | -0.16574 | 0.15954 |
| 0.41932 | -0.36466 | -0.31365 | -0.20125 | 0.19165 |
| 0.49267 | -0.42841 | -0.36778 | -0.23951 | 0.22589 |
| 0.55244 | -0.47934 | -0.41376 | -0.27346 | 0.25608 |
| 0.61571 | -0.53649 | -0.46309 | -0.31063 | 0.28923 |
| 0.65918 | -0.57264 | -0.49457 | -0.33578 | 0.31212 |

Chart 6

SECTION II
Test Strain Data

STRAINS

| " 1T1009" | " 1T1010" | " 1T1011" | " 1T1012" | " 1T1013" | " 2T1009" |
|-----------|-----------|------------|-----------|-----------|-----------|
| " MST " | " MST " | " MST " | " MST " | " MST " | " MST " |
| 0 | -0.26075 | -1.04522 | -1.04236 | -1.04527 | -1.03109 |
| -1.82052 | -7.04018 | -15.15401 | -3.90876 | -12.02065 | 16.75423 |
| -0.26009 | -8.3439 | -21.4245 | -3.38757 | -15.15646 | 27.32236 |
| 1.0403 | -11.47288 | -31.87544 | 0.2607 | -13.58858 | 41.75674 |
| 2.08057 | -14.60185 | -40.49747 | 0.2607 | -17.24705 | 53.35596 |
| 2.86079 | -17.99157 | -47.02927 | 0 | -17.76966 | 56.44922 |
| 1.56043 | -22.16352 | -56.95758 | -0.26048 | -20.64416 | 66.24393 |
| 1.0403 | -26.07473 | -65.3183 | -1.82403 | -26.13184 | 72.9458 |
| 1.0403 | -30.50742 | -71.58899 | -2.08472 | -27.43847 | 76.55438 |
| 0.78021 | -31.81113 | -74.72424 | -3.12688 | -28.74505 | 76.03903 |
| 2.08057 | 1.56446 | -1.56783 | 2.34521 | 3.1358 | 0.77301 |
| -1.56044 | -5.73647 | -29.00151 | 6.77516 | -2.09054 | 42.01483 |
| -4.68134 | -9.90842 | -41.80379 | 5.47231 | -13.58858 | 64.18216 |
| -5.46156 | -14.60185 | -57.2189 | 8.3387 | -17.50835 | 86.86484 |
| -5.72161 | -18.25231 | -70.80508 | 11.72628 | -18.81492 | 108.00107 |
| -5.20148 | -22.42426 | -82.56235 | 14.59268 | -21.16682 | 125.52875 |
| -5.9817 | -25.0317 | -93.79721 | 17.45908 | -21.95078 | 140.9942 |
| -8.32235 | -29.98594 | -106.07709 | 18.76192 | -25.60925 | 155.94434 |
| -8.84251 | -34.41862 | -116.00543 | 20.32547 | -26.6545 | 167.80121 |
| -9.62273 | -37.54759 | -121.49219 | 21.62833 | -25.60925 | 173.98737 |
| 1.0403 | 1.04297 | -3.13545 | 2.8664 | 4.96503 | 2.31984 |
| -4.68134 | -5.99721 | -35.7946 | 8.59919 | -4.70374 | 56.96455 |
| -7.54213 | -10.16916 | -53.56104 | 9.64156 | -14.89515 | 86.34909 |
| -8.06229 | -16.16631 | -74.72424 | 16.6772 | -15.41781 | 120.37329 |
| -12.22346 | -21.12055 | -95.88724 | 21.36784 | -19.33758 | 155.42859 |
| -13.52382 | -25.81399 | -110.51862 | 25.01591 | -21.95078 | 176.56488 |
| -16.6447 | -30.76817 | -126.71747 | 28.14279 | -25.87054 | 200.79437 |
| -19.24542 | -35.98308 | -138.99731 | 30.7487 | -28.74505 | 219.35309 |
| -22.88646 | -41.4588 | -151.7998 | 33.09392 | -30.83558 | 234.30322 |
| -23.92676 | -44.06628 | -161.20551 | 33.87558 | -29.529 | 244.61328 |
| -8.58243 | -3.91121 | -6.532 | -6.25397 | -3.91977 | -4.63969 |
| -15.08426 | -13.03739 | -48.33557 | -0.52118 | -15.94042 | 63.66641 |
| -18.98537 | -18.51305 | -71.3277 | 5.21162 | -22.73473 | 100.78387 |
| -21.84616 | -25.0317 | -97.45505 | 13.02914 | -24.56393 | 146.14923 |
| -24.4469 | -29.98594 | -120.18567 | 20.84666 | -27.69978 | 186.87531 |
| -27.82787 | -34.94011 | -141.08752 | 27.8823 | -29.00634 | 220.89954 |
| -28.86815 | -39.89429 | -154.41241 | 34.13628 | -28.48374 | 244.61328 |
| -31.72896 | -43.8055 | -170.35016 | 37.26315 | -32.9261 | 268.84277 |
| -35.63007 | -50.32413 | -185.24268 | 41.43262 | -36.58456 | 291.01013 |
| -39.01102 | -53.71387 | -195.43231 | 43.77783 | -36.84592 | 305.44458 |
| -16.90477 | -8.60464 | -12.54118 | -15.63483 | -9.40746 | -6.95953 |
| -23.40663 | -18.25231 | -61.92175 | -0.78167 | -17.76966 | 80.67871 |
| -26.78757 | -23.98873 | -88.0491 | 5.47231 | -26.91581 | 127.07516 |
| -30.42859 | -29.98594 | -118.35675 | 15.63504 | -29.79031 | 178.62701 |

Chart 1

STRAINS

| | | | | | |
|-----------|-----------|------------|-----------|-----------|-----------|
| -35.36998 | -36.50456 | -144.74542 | 25.27639 | -29.79031 | 226.57031 |
| -39.53116 | -41.4588 | -167.4762 | 33.6151 | -31.09689 | 264.71887 |
| -42.65205 | -47.71071 | -187.85547 | 41.43262 | -35.278 | 301.83594 |
| -46.033 | -52.93167 | -207.45105 | 48.72894 | -37.36853 | 337.40662 |
| -50.19417 | -58.66803 | -222.86597 | 53.41937 | -39.19777 | 361.63611 |
| -53.31505 | -62.05774 | -233.31689 | 54.46175 | -41.2883 | 376.07043 |
| -23.40663 | -14.60185 | -18.55057 | -20.06478 | -11.49805 | -7.73294 |
| -30.42859 | -23.20649 | -71.58899 | -6.51446 | -27.43847 | 90.98877 |
| -34.32968 | -28.42142 | -105.55447 | 6.25397 | -29.79031 | 152.33575 |
| -39.53116 | -35.46159 | -141.34888 | 20.58596 | -32.4035 | 218.83734 |
| -43.69232 | -41.198 | -170.87274 | 33.09392 | -31.88085 | 273.48242 |
| -50.45424 | -48.23819 | -200.13538 | 44.03831 | -35.278 | 328.12756 |
| -54.61543 | -53.45316 | -221.82074 | 52.11632 | -37.62984 | 366.7915 |
| -57.73631 | -59.45032 | -240.37134 | 60.9762 | -37.89119 | 400.29993 |
| -62.41762 | -64.66528 | -259.70532 | 68.79373 | -39.98172 | 435.87061 |
| -64.75827 | -67.27274 | -270.67896 | 74.00534 | -39.72043 | 455.46045 |
| -24.70699 | -15.12334 | -19.07297 | -21.10714 | -11.49805 | -8.50595 |
| -32.76926 | -24.77096 | -82.56235 | -1.82403 | -27.17712 | 111.60968 |
| -37.45058 | -30.50742 | -120.18567 | 12.76844 | -29.79031 | 183.78204 |
| -40.57146 | -37.02606 | -160.16052 | 32.31224 | -28.48374 | 261.62561 |
| -45.77293 | -41.71954 | -195.17102 | 48.46826 | -30.05161 | 331.22046 |
| -49.67403 | -47.71671 | -221.55988 | 63.06093 | -30.05161 | 386.38098 |
| -55.13557 | -53.19235 | -247.16443 | 74.26584 | -30.05161 | 436.38635 |
| -59.81688 | -57.88586 | -269.89502 | 85.47092 | -32.4035 | 479.17419 |
| -65.79855 | -63.10074 | -290.27466 | 93.80963 | -33.44876 | 518.354 |
| -68.91946 | -66.49045 | -304.38306 | 100.06342 | -34.23273 | 543.61426 |

Chart 2

STRAINS

| " 2T1010" | " 2T1011" | " 2T1012" | " 2T1013" | " 3T1009" | " 3T1010" |
|-----------|------------|-----------|-----------|-----------|-----------|
| " MST " | " MST " | " MST " | " MST " | " MST " | " MST " |
| -1.54683 | 1.02696 | -4.36252 | -3.86532 | 0 | -2.58828 |
| 4.64089 | -18.99861 | -4.10595 | -6.18445 | 19.06746 | 10.87062 |
| 6.18772 | -33.11916 | -2.82282 | -5.15372 | 29.77654 | 18.37659 |
| 6.96113 | -50.3205 | 21.29935 | 10.82273 | 43.09766 | 25.6237 |
| 8.76603 | -70.08929 | 29.76782 | 18.29559 | 54.85156 | 32.09435 |
| -1.54683 | -91.91193 | 44.65173 | 29.89143 | 57.72469 | 28.98846 |
| -0.25767 | -112.7077 | 52.09363 | 37.36429 | 68.95618 | 36.49443 |
| -5.15623 | -133.24664 | 53.12021 | 39.94116 | 76.53094 | 39.60033 |
| -16.24252 | -156.09631 | 68.51733 | 51.53697 | 80.18777 | 38.04738 |
| -23.97706 | -172.01404 | 72.87985 | 57.9791 | 80.18777 | 34.94148 |
| 1.03149 | -4.10786 | 7.95514 | 6.44213 | 0.5224 | 0 |
| 37.64165 | -14.89074 | 35.41347 | 17.00719 | 43.09766 | 33.64729 |
| 55.68907 | -24.64677 | 27.97146 | 5.66906 | 66.34427 | 53.31808 |
| 73.99417 | -37.22691 | 45.16496 | 13.14192 | 90.37445 | 72.47113 |
| 86.3692 | -51.0907 | 64.66806 | 23.19164 | 111.27023 | 86.96536 |
| 96.16632 | -67.26517 | 80.83499 | 33.7567 | 130.07648 | 100.16541 |
| 102.61172 | -84.20984 | 100.85132 | 45.09483 | 144.44244 | 109.74203 |
| 110.34668 | -101.41116 | 109.57635 | 50.24855 | 162.46509 | 121.64798 |
| 113.18265 | -119.38281 | 126.76984 | 61.84436 | 174.74127 | 128.37738 |
| 112.92459 | -132.73322 | 144.47662 | 73.95554 | 180.48773 | 131.4834 |
| 9.0237 | -1.28373 | 19.24643 | 13.14192 | 2.08962 | 5.69417 |
| 63.16554 | -8.47239 | 35.41347 | 12.11118 | 59.8143 | 53.83569 |
| 91.0101 | -16.43124 | 39.77599 | 6.69979 | 89.32965 | 78.94177 |
| ***** | -24.64677 | 73.90631 | 22.1609 | 123.02423 | 105.85956 |
| 154.1756 | -35.17299 | 95.97556 | 31.17983 | 159.33069 | 134.33044 |
| 170.16046 | -48.52333 | 117.53159 | 41.22957 | 181.27124 | 151.15405 |
| 190.78577 | -62.64388 | 135.75159 | 49.73318 | 207.39105 | 171.86005 |
| 202.64551 | -78.56158 | 149.86566 | 56.94836 | 226.98096 | 186.09552 |
| 212.95837 | -94.73605 | 166.54584 | 66.22501 | 243.95874 | 198.26038 |
| 220.17712 | -106.03247 | 189.64154 | 80.65536 | 255.19037 | 206.80157 |
| 11.08628 | -2.05393 | 5.64565 | 5.92673 | -1.30601 | 10.09415 |
| 80.43954 | -5.39149 | 31.82075 | 4.896 | 68.95618 | 67.29459 |
| 119.62805 | -9.24258 | 48.75768 | 6.95747 | 108.13589 | 100.68311 |
| 166.55103 | -14.89074 | 87.50708 | 21.90318 | 153.06201 | 138.73047 |
| 207.28638 | -22.8496 | 114.96542 | 32.21062 | 194.33118 | 173.15424 |
| 240.80286 | -32.60562 | 141.65375 | 42.51802 | 230.63776 | 202.1427 |
| 258.84998 | -45.18578 | 173.47461 | 58.23677 | 253.10077 | 220.00171 |
| 281.53821 | -59.30632 | 188.61511 | 64.16351 | 279.22058 | 241.48419 |
| 301.13245 | -74.45383 | 203.49908 | 72.15176 | 304.29553 | 260.89612 |
| 312.99219 | -83.69641 | 219.92273 | 81.6861 | 319.70618 | 273.8374 |
| 17.53167 | -1.79716 | 3.59262 | 4.63833 | -2.87323 | 13.97661 |
| 110.862 | 2.05383 | 47.98778 | 11.85346 | 87.24002 | 90.07126 |
| 161.91016 | 0.7702 | 64.41138 | 9.53432 | 134.77814 | 130.44806 |
| 218.63031 | -3.33756 | 102.90433 | 22.1609 | 188.5849 | 176.00128 |

Chart 3

STRAINS

| | | | | | |
|-----------|-----------|-----------|-----------|-----------|-----------|
| 269.16284 | -8.21562 | 148.58252 | 40.71422 | 236.38403 | 217.15454 |
| 309.38232 | -16.43124 | 182.71295 | 55.14456 | 276.60852 | 250.28412 |
| 348.05542 | -25.1603 | 201.44617 | 57.9791 | 315.52698 | 283.67273 |
| 385.1814 | -34.65955 | 227.62128 | 69.57492 | 352.09473 | 315.76709 |
| 409.41614 | -49.03687 | 248.15076 | 78.07852 | 377.95337 | 336.99084 |
| 422.82263 | -59.04955 | 258.41565 | 83.74759 | 393.62512 | 349.67334 |
| 21.39915 | -0.7702 | 11.80442 | 10.56506 | -1.82841 | 18.11774 |
| ***** | 7.18866 | 40.54578 | 0.77306 | 99.25513 | 107.15375 |
| 205.22382 | 10.26945 | 80.32175 | 11.59579 | 164.03229 | 162.28357 |
| 279.47559 | 12.06662 | 127.79642 | 26.54151 | 229.85406 | 221.29578 |
| 338.77368 | 8.47229 | 178.09381 | 45.61021 | 285.75049 | 267.88452 |
| 402.71301 | 5.13473 | 210.68433 | 55.40224 | 342.4303 | 318.09644 |
| 447.05774 | -2.56736 | 240.45215 | 64.93658 | 383.96094 | 355.10852 |
| 481.60535 | -12.58015 | 275.60925 | 81.17075 | 418.17773 | 384.09692 |
| 523.8877 | -22.59293 | 295.11218 | 86.32443 | 456.05139 | 420.8501 |
| 547.09155 | -30.03825 | 314.61511 | 94.82803 | 477.20837 | 441.03857 |
| 23.71939 | 1.28363 | 8.9816 | 7.9882 | -1.82841 | 22.25896 |
| 165.00385 | 15.91771 | 54.40323 | 3.34987 | 121.457 | 132.25989 |
| 249.56854 | 22.8496 | 96.74545 | 13.65726 | 193.54767 | 196.44855 |
| 338.77368 | 27.72755 | 161.41351 | 34.52977 | 272.95166 | 266.3313 |
| 422.30725 | 28.24109 | 206.06512 | 47.41402 | 343.99756 | 331.29663 |
| 486.24622 | 25.67373 | 251.48676 | 62.35973 | 399.89392 | 381.24988 |
| 547.60693 | 21.82263 | 287.92676 | 75.75934 | 452.65576 | 432.49731 |
| 598.65503 | 14.37721 | 320.51746 | 86.32443 | 497.58191 | 472.87402 |
| 645.57812 | 5.64816 | 348.74548 | 96.11646 | 539.89575 | 511.18018 |
| 676.51636 | -0.51353 | 367.22217 | 102.81628 | 565.49316 | 535.76855 |

Chart 4

STRAINS

| " 3T1011" | " 3T1012" | " 3T1013" | " L 1" |
|------------|-----------|-----------|-----------|
| " MST " | " MST " | " MST " | " LBS " |
| -0.26059 | -4.42657 | -1.30672 | 0.24346 |
| 16.15704 | -17.70631 | 0.2613 | 17.67105 |
| 31.79279 | -24.73676 | 2.35215 | 29.844 |
| 56.54953 | -16.92516 | 9.66988 | 44.89787 |
| 78.70032 | -20.0498 | 13.59006 | 59.58656 |
| 111.53552 | -15.36283 | 18.81716 | 74.62012 |
| 133.68622 | -21.35174 | 23.26015 | 89.75516 |
| 158.18231 | -30.46528 | 26.3962 | 104.56558 |
| 191.53876 | -26.29908 | 31.3618 | 119.78174 |
| 213.42889 | -27.60102 | 34.49806 | 129.56073 |
| 10.16325 | 2.0831 | 5.74971 | 0.3449 |
| 0 | 5.20774 | 8.36316 | 17.69135 |
| -3.90896 | -13.54012 | 7.57904 | 30.00631 |
| 0.26059 | -13.54012 | 10.71531 | 44.71529 |
| 10.42384 | -11.71741 | 15.4196 | 59.7083 |
| 22.41132 | -9.89471 | 20.12389 | 74.64041 |
| 38.82895 | -5.46812 | 25.08948 | 89.71457 |
| 50.81644 | -11.45702 | 29.00965 | 104.84961 |
| 68.27643 | -9.37393 | 33.71394 | 119.78174 |
| 85.47583 | 0 | 37.63412 | 129.76361 |
| 8.07852 | 11.45703 | 8.36316 | 0.3449 |
| -20.84776 | 0.78116 | 8.36316 | 17.71163 |
| -34.65938 | -11.45702 | 8.10186 | 29.92515 |
| -38.82893 | -2.0831 | 13.85157 | 44.8573 |
| -47.68922 | -1.04155 | 16.46502 | 59.58656 |
| -41.69554 | 2.34348 | 21.69192 | 74.66071 |
| -41.69554 | 1.82271 | 24.82818 | 89.75516 |
| -34.91998 | -0.26039 | 28.22574 | 104.76846 |
| -24.23555 | 1.04155 | 32.14592 | 119.94406 |
| -14.07221 | 10.93625 | 36.06609 | 130.33167 |
| 4.95133 | 0.26039 | 4.18168 | 0.3449 |
| -41.17436 | -8.33238 | 3.39757 | 17.73192 |
| -63.58566 | -11.71741 | 4.44299 | 29.98602 |
| -81.04565 | -1.56232 | 9.40858 | 44.79643 |
| -96.94211 | 0.26039 | 14.11287 | 59.85031 |
| -106.32358 | 4.94735 | 17.77174 | 74.76215 |
| -101.89343 | 17.18554 | 25.08948 | 89.55228 |
| -104.49936 | 13.27974 | 27.96423 | 104.68729 |
| -104.75995 | 10.67587 | 31.10049 | 119.86292 |
| -101.63284 | 14.0609 | 33.97525 | 129.96649 |
| -1.82413 | -3.90581 | 1.56803 | 0.3449 |
| -65.67041 | -2.34348 | 4.18168 | 17.71163 |
| -98.50565 | -9.11354 | 3.65887 | 30.10775 |
| -128.4743 | -1.82271 | 8.36316 | 44.69499 |

Chart 5

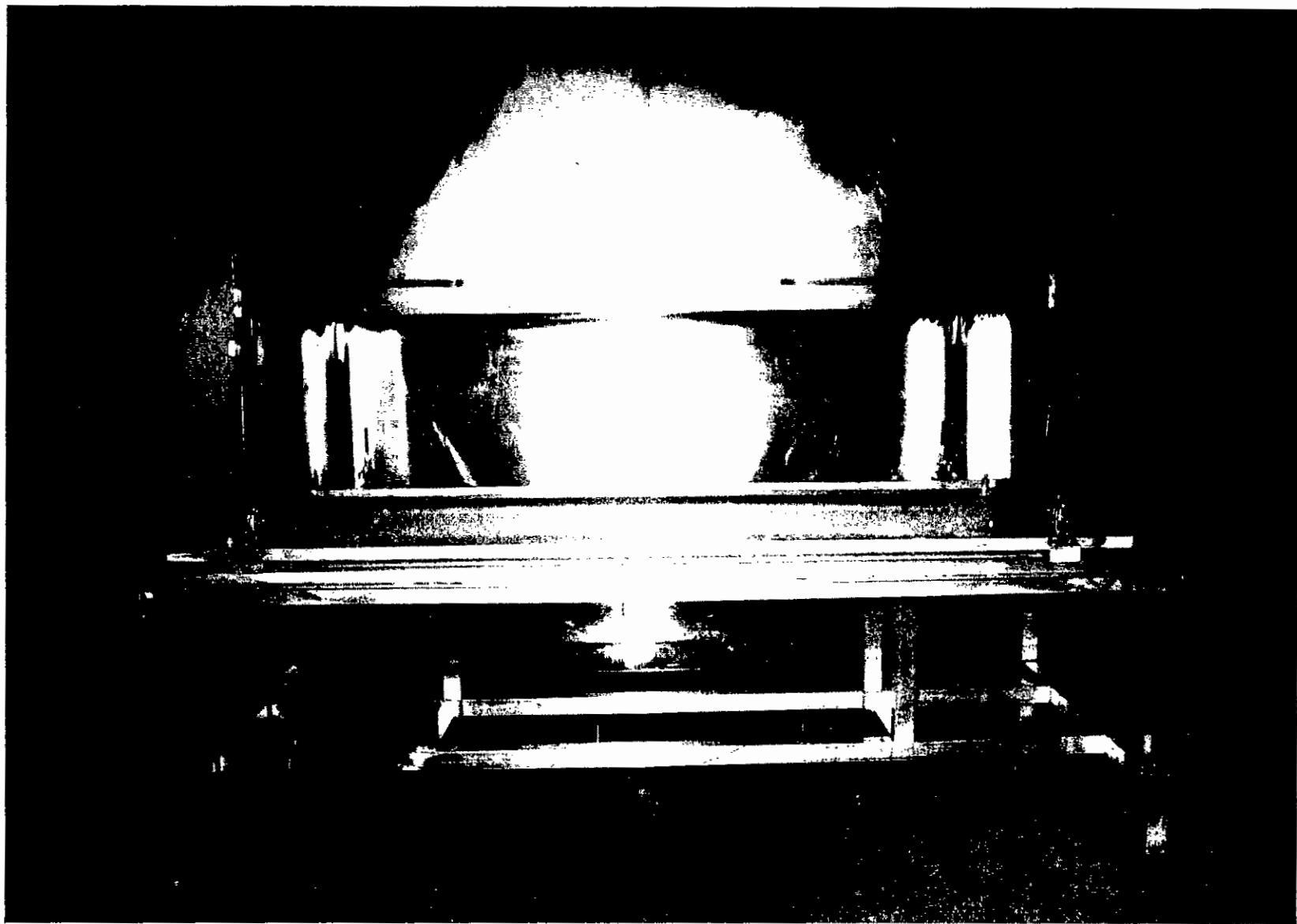
STRAINS

| | | | |
|------------|-----------|----------|-----------|
| -149.5827 | 13.80051 | 14.37418 | 59.80974 |
| -163.65491 | 22.91415 | 19.07847 | 74.51868 |
| -182.93903 | 18.48758 | 22.21473 | 89.71457 |
| -196.49011 | 20.83106 | 26.91901 | 104.60614 |
| -200.39911 | 22.39338 | 30.31638 | 119.70059 |
| -200.65961 | 21.61222 | 32.14592 | 129.64185 |
| -4.16955 | 1.56232 | 1.56803 | 0.36519 |
| -90.68774 | -13.54012 | 0 | 17.73192 |
| -135.77106 | -5.9889 | 2.87496 | 29.844 |
| -178.76953 | 4.68697 | 8.10186 | 44.69499 |
| -209.5199 | 22.39338 | 14.63548 | 59.99232 |
| -247.04596 | 23.69531 | 16.98763 | 74.86359 |
| -266.85132 | 28.64267 | 21.95322 | 89.59286 |
| -277.01465 | 39.83931 | 27.18032 | 104.84961 |
| -297.08069 | 38.0166 | 30.8392 | 119.66003 |
| -303.33484 | 42.18279 | 34.23676 | 129.8042 |
| -5.7331 | -0.52077 | 0.52281 | 0.30432 |
| -114.66269 | -7.81161 | 1.04542 | 17.79279 |
| -171.47284 | -2.0831 | 3.92017 | 29.78313 |
| -226.4588 | 19.52912 | 10.97661 | 45.18192 |
| -276.49353 | 27.60112 | 15.9422 | 59.74887 |
| -311.67395 | 40.09969 | 22.21473 | 75.02591 |
| -346.07275 | 48.43207 | 26.1349 | 89.75516 |
| -370.82959 | 54.94174 | 31.3618 | 104.64673 |
| -394.80444 | 57.806 | 34.75937 | 119.49771 |
| -408.87671 | 60.14949 | 37.89563 | 130.29108 |

Chart 6

SECTION III

Manufacturing and Testing Photographs



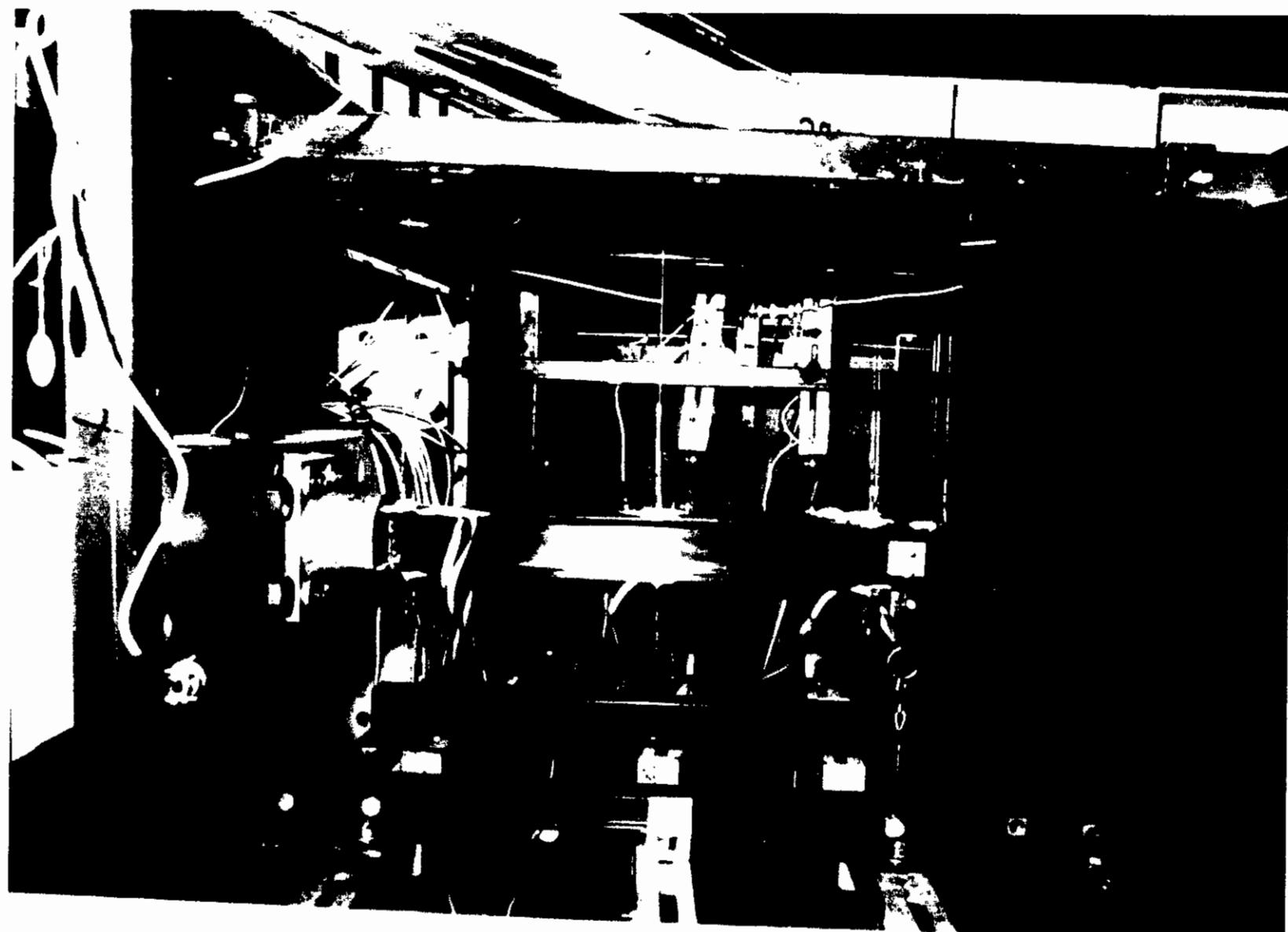
Hot-drape forming "oven."



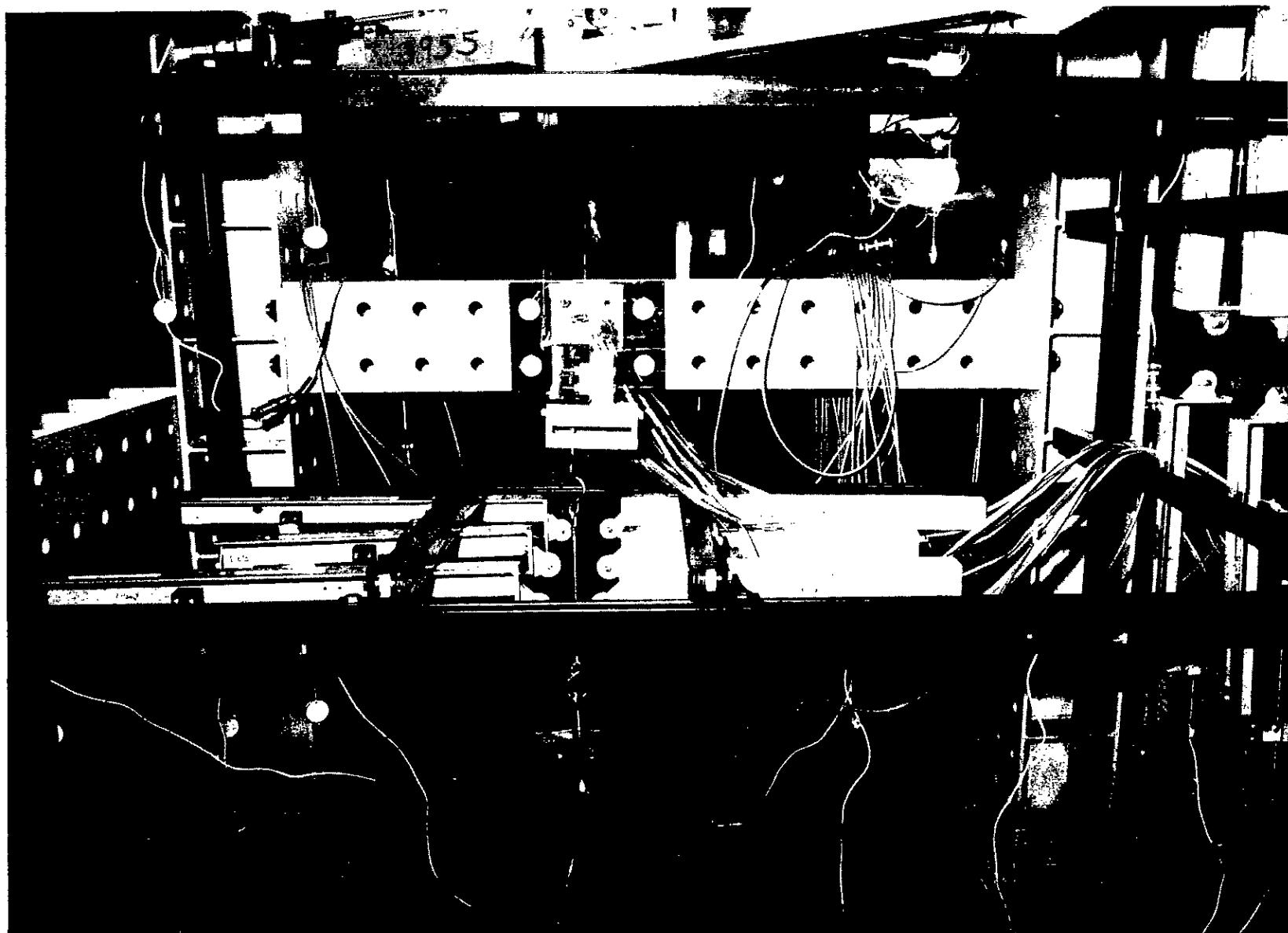
5
Laminate placed on tool ready for hot-drape forming.



C-channel upon removal from tool after autoclave cure.
Notice warpage of beam along its length.



Fully instrumented beam C-channel prior to testing.



Front view of test configuration. Slide block allows load application point to be changed.



C-channel with load being applied. No rotation has occurred at this point of the test.



Test being performed on L-angle. Beam has started to rotate.

| REPORT DOCUMENTATION PAGE | | | Form Approved OMB No. 0704-0188 |
|--|--|---|--|
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| 1. AGENCY USE ONLY (Leave blank) | 2. REPORT DATE | 3. REPORT TYPE AND DATES COVERED | |
| | October 1994 | Technical Paper | |
| 4. TITLE AND SUBTITLE Buckling of Composite Beams (CDDF Final Report - Project #91-20) | | | 5. FUNDING NUMBERS |
| 6. AUTHOR(S) P. Thompson | | | |
| 7. PERFORMING ORGANIZATION NAME(S) AND ADDRESS(ES) George C. Marshall Space Flight Center Marshall Space Flight Center, Alabama 35812 | | | 8. PERFORMING ORGANIZATION REPORT NUMBER M-765 |
| 9. SPONSORING/MONITORING AGENCY NAME(S) AND ADDRESS(ES) National Aeronautics and Space Administration Washington, DC 20546 | | | 10. SPONSORING/MONITORING AGENCY REPORT NUMBER NASA TP-3512 |
| 11. SUPPLEMENTARY NOTES Prepared by Structures and Dynamics Laboratory, Science and Engineering Directorate. | | | |
| 12a. DISTRIBUTION/AVAILABILITY STATEMENT Subject Category: 24 Unclassified-Unlimited | | | 12b. DISTRIBUTION CODE |
| 13. ABSTRACT (Maximum 200 words) Presented in this report are the results of an investigation of the twisting/warping deformations occurring in open-section composite beams. A series of C and L channels were manufactured using both hand layup and the innovative "hot-drape forming" techniques. A transverse tip load was applied at the free end of the cantilevered open-section beams. The test setup allowed the tip load to be applied at various locations along the plane of and at the beam's shear center. Charts are included in this report depicting various angles of ply layups, loads applied, and load application points. | | | |
| A major verification resulting from this study is that the shear center of an open section composite beam can be altered, if not completely controlled, through laminate layup. Also, it was observed that the choice of the material system does not have an effect on the amount of deformation, as expected, and the material affects the location of an unsymmetric open section composite beam's true shear center. The results from this study have provided a foundation for further investigation into the apparent shifting of the shear center location in open-section composite beams. | | | |
| 14. SUBJECT TERMS antisymmetric laminate layup, hot-drape forming, torsion/bending coupling, composite beams | | | 15. NUMBER OF PAGES 54 |
| | | | 16. PRICE CODE A04 |
| 17. SECURITY CLASSIFICATION OF REPORT Unclassified | 18. SECURITY CLASSIFICATION OF THIS PAGE Unclassified | 19. SECURITY CLASSIFICATION OF ABSTRACT Unclassified | 20. LIMITATION OF ABSTRACT Unlimited |

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